

# CONSORZIO DI BONIFICA STORNARA E TARA

viale Magna Grecia, 240 - 74121 TARANTO

*"RIPRISTINO DEL PONTE TUBO DELL'IMPIANTO IRRIGUO CONSORTILE  
SX BRADANO UBICATO IN ATTRAVERSAMENTO DELLA LAMA DI LATERZA"*  
COMUNE DI CASTELLANETA (TARANTO)

## PROGETTO DEFINITIVO

CIG 7845120DD0



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ELABORATO	DATA	SCALA	ALLEGATO
Verifica strutturale di ripristino delle selle d'appoggio, catene e travi	02/2021	-	R.7.s.4

AGGIORNAMENTO	DATA	DESCRIZIONE

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N.B. In azzurro sono evidenziate le zone e le armature di rinforzo con FRCC in PBO.

## VERIFICA SELLE DI TESTA

### PARAMETRI GENERALI

Metodo di verifica : stati limite - NTC18 ->  
 Duttilita' : non prevista (struttura non dissipativa)  
 Unita' di misura : cm; kN; kN/m; kNm; N/mm2; deform. %.  
 Unita' particolari : fessure [wk]:mm - ferri:mm e cm2 - sezioni:cm e derivate.  
 Copriferri (assi) : longitudinali= 3 ; staffe= 2

### CARATTERISTICHE MATERIALI

CLS : Rck =30. ; fck=24.9 ; fctk= 1.79; fctm= 2.56; Ec= 31447.2 ;  
 gc =1.5 ; fcd=14.11; fbd= 2.69; fctd= 1.19; Ecu=.2% (limit.elastico)  
 ACCIAIO : B450C; ftk=517.5 ; fyk=450. ; Es=210000. ;  
 gs =1.15; fyd=391.3 ; ftd(k\*fyd)=450. ; fud=443.98; Eud=.19% (limit.elastico)

### VALORI TENSIONI MASSIME IN ESERCIZIO

GRUPPO : aggressivo.

CLS :  $\sigma_c$  (rara)=14.94;  $\sigma_c$  (quasi permanente)=11.2 ; fbd(esercizio)= 2.69  
 ACCIAIO :  $\sigma_f$  (rara)=360. ; Coeff.Omogeneizzazione= 15  
 FESSURE : wmax(fre.)=.3 ; wmax(q.p.)=.2 [4.1.2.2.4.5]; kt=.4 [EN 1992-1 7.3.4].

### CASI DI CARICO

SLU		
Nome	Descrizione	Sest
1.	SLU	1.
2.	SLU VENTYOY	2.
6.	SLU con SISMAX PRINC32	
7.	SLU con SISMAX PRINC32	
8.	SLU con SISMAZ PRINC32	
13.	SLU Solo Perm.	1.

RARE			FREQUENTI			QUASI PERMANENTI		
Nome	Descrizione	Sest	Nome	Descrizione	Sest	Nome	Descrizione	Sest
16.	Rara	1.	18.	Frequente	1.	20.	Quasi Perm	1.
17.	Rara Ventoy	2.	19.	Frequente Ventoy	2.			

### SEZIONI UTILIZZATE

1) Rettangolare: 40x60; A=2400.; Jg=720000.; E=31447.16

### ARMATURE LONGITUDINALI (%=100\*Af/Acl - Acl=area intera sezione)

Nro	Totale	%	Super.	%	Barre	Infer.	%	Barre
1	13.45	.56	9.42	.393	3d20	4.02	.168	2d16
2	22.87	.953	18.85	.785	3d20 +3d20	4.02	.168	2d16

## SELLA TIPO:

### DESCRIZIONE CAMPATE

Cam.	Descriz.	S.ini	Sez.	S.fin	Incl.	L.assi	L.net.	lambda	K	r.Ar.	lam.max
1	A101-A132	1	1	1	0	304.	274.	5.073	1.	2.967	109.768

### VERIFICHE ALLO STATO LIMITE ULTIMO

#### FLESSIONE:

Progressive	Se	Ar	Msd	Epsc	Epsac	Mrd	Epsc	Epsac	Cam	x/d	Mr/Ms	Ve		
>	0.	0.	3.	1.	-14.3332	-.005	.014	-190.4466	-.077	.186	2.	.292	13.29	SI
	0.	0.	3.	1.	4.8946	-.002	.011	84.59	-.039	.186	2.	.174	17.28	SI
	114.	114.	3.	1.	26.2339	-.012	.058	84.59	-.039	.186	2.	.174	3.224	SI
	152.	152.	3.	2.	-4.8805	-.001	.002	-364.9489	-.126	.186	2.	.403	74.78	SI
	152.	152.	3.	2.	28.6665	-.011	.063	85.0661	-.033	.186	2.	.149	12.967	SI
	304.	304.	3.	1.	-14.3332	-.005	.014	-190.4466	-.077	.186	2.	.292	13.29	SI
	304.	304.	3.	1.	4.8946	-.002	.011	84.59	-.039	.186	2.	.174	17.28	SI

#### TAGLIO:

Progressive	Se	Vsd	VRd	VRcd	VRsd	Asw	s	ctgT	Ve		
>	0.	0.	3.	37.72	80.01	499.2	394.15	1.57	20.	2.5	SI
	304.	304.	3.	-37.72	80.01	499.2	394.15	1.57	20.	2.5	SI

### VERIFICHE ALLO STATO LIMITE DI ESERCIZIO

#### TENSIONI DI ESERCIZIO E FESSURAZIONE - RARE:

Progressive	Se	Ar	Momento	$\sigma_c$	$\sigma_f$	As	hc,ef	Eps%	Sr,max	wd	Ve		
>	0.	0.	3.	1.	-10.0188	-.53	20.45	9.42	7.5	.0058	57.29	.033	SI
	152.	152.	3.	2.	20.0376	-1.08	92.15	4.02	7.5	.0263	66.89	.176	SI
	304.	304.	3.	1.	-10.0188	-.53	20.45	9.42	7.5	.0058	57.29	.033	SI
	304.	304.	3.	1.	.0509	0.	.24	4.02	7.5	.0001	65.2	0.	SI

#### TENSIONI DI ESERCIZIO E FESSURAZIONE - FREQUENTI:

Progressive	Se	Ar	Momento	$\sigma_c$	$\sigma_f$	As	hc,ef	Eps%	Sr,max	wd	Ve		
>	0.	0.	3.	1.	-9.4396	-.5	19.27	9.42	7.5	.0055	57.29	.032	SI
	152.	152.	3.	2.	18.8794	-1.02	86.82	4.02	7.5	.0248	66.89	.166	SI
	304.	304.	3.	1.	-9.4396	-.5	19.27	9.42	7.5	.0055	57.29	.032	SI
	304.	304.	3.	1.	.0102	0.	.05	4.02	7.5	0.	65.2	0.	SI

#### TENSIONI DI ESERCIZIO E FESSURAZIONE - QUASI PERMANENTI:

Progressive	Se	Ar	Momento	$\sigma_c$	$\sigma_f$	As	hc,ef	Eps%	Sr,max	wd	Ve		
	15.	15.	3.	1.	-7.1141	-.38	14.52	9.42	7.5	.0041	57.29	.024	SI
	152.	152.	3.	2.	17.7211	-.96	81.49	4.02	7.5	.0233	66.89	.156	SI
	304.	304.	3.	1.	-8.8606	-.47	18.09	9.42	7.5	.0052	57.29	.03	SI

## VERIFICA SELLE INTERMEDIE:

### PARAMETRI GENERALI

Metodo di verifica : stati limite - NTC18 ->  
 Duttilita' : non prevista (struttura non dissipativa)  
 Unita' di misura : cm; kN; kN/m; kNm; N/mm2; deform. %.  
 Unita' particolari : fessure [wk]:mm - ferri:mm e cm2 - sezioni:cm e derivate.  
 Copriferri (assi) : longitudinali= 3 ; staffe= 2

### CARATTERISTICHE MATERIALI

CLS : Rck=30. ; fck=24.9 ; fctk= 1.79; fctm= 2.56; Ec= 31447.2 ;  
 gc =1.5 ; fcd=14.11; fbd= 2.69; fctd= 1.19; Ecu=.2% (limit.elastico)  
 ACCIAIO : B450C; ftk=517.5 ; fyk=450. ; Es=210000. ;  
 gs =1.15; fyd=391.3 ; ftd(k\*fyd)=450. ; fud=443.98; Eud=.19% (limit.elastico)

### VALORI TENSIONI MASSIME IN ESERCIZIO

GRUPPO : aggressivo.  
 CLS :  $\sigma_c$  (rara)=14.94;  $\sigma_c$  (quasi permanente)=11.2 ; fbd(esercizio)= 2.69  
 ACCIAIO :  $\sigma_f$  (rara)=360. ; Coeff.Omogeneizzazione= 15  
 FESSURE : wmax(fre.)=.3 ; wmax(q.p.)=.2 [4.1.2.2.4.5]; kt=.4 [EN 1992-1 7.3.4].

### CASI DI CARICO

SLU			FREQUENTI			QUASI PERMANENTI		
Nome	Descrizione	Sest	Nome	Descrizione	Sest	Nome	Descrizione	Sest
1.	SLU	1.	16.	Rara	1.	20.	Quasi Perm	1.
2.	SLU VENTYO	2.	17.	Rara VentoY	2.	19.	Frequente VentoY	2.
6.	SLU con SISMAX	PRINC32						
7.	SLU con SISMAX	PRINC32						
8.	SLU con SISMAZ	PRINC32						
13.	SLU Solo Perm.	1.						

### SEZIONI UTILIZZATE

2) Rettangolare: 25X70; A=1750.; Jg=714583.; E=31447.16

### ARMATURE LONGITUDINALI (%=100\*Af/Acl - Acl=area intera sezione)

Nro	Totale	% Super.	% Infer.	Barre	Barre
1	12.06	.689	4.02	.23   2d16	8.04   .46   4d16

## SELLA TIPO: I intermedia

### DESCRIZIONE CAMPATE

Cam.	Descriz.	S.ini	Sez.	S.fin	Incl.	L.assi	L.net.	lambda	K	r.Ar.	lam.max
1	A201-A280	2	2	2	0	304.	304.	4.349	1.	2.295	41.863

### VERIFICHE ALLO STATO LIMITE ULTIMO

#### FLESSIONE:

Progressive	SE	Ar	Msd	Epsc	Epsac	Mrd	Epsc	Epsac	Cam	x/d	Mr/Ms	VE
> 15.	15.	3.	11.1447	0.	0.	-99.2562	-0.046	.186	2.	.197	2375.	SI
15.	15.	3.	24.7535	-.01	.024	190.9458	-.082	.186	2.	.306	7.714	SI
152.	152.	3.	83.207	-.034	.081	190.9458	-.082	.186	2.	.306	2.295	SI
274.	274.	3.	-.0381	0.	0.	-99.2562	-.046	.186	2.	.197	2606.	SI

#### TAGLIO:

Progressive	Se	Vsd	VRd	VRcd	VRsd	Asw	s	ctgT	Ve
> 0.	0.	3.	109.36	56.25	366.74	237.21	1.01	25.	2.5
70.	70.	3.	59.08	71.07	366.74	237.21	1.01	25.	2.5
304.	304.	3.	-109.36	56.25	377.59	379.53	1.01	15.	2.4

### VERIFICHE ALLO STATO LIMITE DI ESERCIZIO

#### TENSIONI DI ESERCIZIO E FESSURAZIONE - RARE:

Progressive	Se	Ar	Momento	$\sigma_c$	$\sigma_f$	As	hc,ef	Eps%	Sr,max	wd	Ve
30.	30.	3.	11.1447	-.62	22.7	8.04	7.5	.0065	13.82	.009	SI
152.	152.	3.	56.147	-3.15	114.35	8.04	7.5	.0398	13.82	.055	SI
289.	289.	3.	5.584	-.31	11.37	8.04	7.5	.0032	13.82	.004	SI

#### TENSIONI DI ESERCIZIO E FESSURAZIONE - FREQUENTI:

Progressive	Se	Ar	Momento	$\sigma_c$	$\sigma_f$	As	hc,ef	Eps%	Sr,max	wd	Ve
30.	30.	3.	10.2295	-.57	20.83	8.04	7.5	.006	13.82	.008	SI
152.	152.	3.	51.6173	-2.89	105.13	8.04	7.5	.0354	13.82	.049	SI
289.	289.	3.	5.1171	-.29	10.42	8.04	7.5	.003	13.82	.004	SI

#### TENSIONI DI ESERCIZIO E FESSURAZIONE - QUASI PERMANENTI:

Progressive	Se	Ar	Momento	$\sigma_c$	$\sigma_f$	As	hc,ef	Eps%	Sr,max	wd	Ve
30.	30.	3.	9.3279	-.52	19.	8.04	7.5	.0054	13.82	.008	SI
152.	152.	3.	47.0883	-2.64	95.9	8.04	7.5	.0311	13.82	.043	SI
289.	289.	3.	4.6639	-.26	9.5	8.04	7.5	.0027	13.82	.004	SI

## SELLA TIPO: II intermedia

### DESCRIZIONE CAMPATE

Cam.	Descriz.	S.ini	Sez.	S.fin	Incl.	L.assi	L.net.	lambda	K	r.Ar.	lam.max
1	A201-A280	2	2	2	0	304.	304.	4.349	1.	1.759	32.083

### VERIFICHE ALLO STATO LIMITE ULTIMO

#### FLESSIONE:

Progressive	SE	Ar	Msd	Epsc	Epsac	Mrd	Epsc	Epsac	Cam	x/d	Mr/Ms	VE
> 15.	15.	3.	-.0322	0.	0.	-99.2562	-.046	.186	2.	.197	3084.	SI
15.	15.	3.	32.2835	-.013	.031	190.9458	-.082	.186	2.	.306	5.915	SI
152.	152.	3.	108.5707	-.045	.106	190.9458	-.082	.186	2.	.306	1.759	SI
274.	274.	3.	-.0322	0.	0.	-99.2562	-.046	.186	2.	.197	3086.	SI

#### TAGLIO:

Progressive	Se	Vsd	VRd	VRcd	VRsd	Asw	s	ctgT	Ve
> 0.	0.	3.	142.69	56.25	366.74	237.21	1.01	25.	2.5
70.	70.	3.	77.07	71.07	366.74	237.21	1.01	25.	2.5
304.	304.	3.	-142.69	56.25	377.59	379.53	1.01	15.	2.4

### VERIFICHE ALLO STATO LIMITE DI ESERCIZIO

#### TENSIONI DI ESERCIZIO E FESSURAZIONE - RARE:

Progressive	Se	Ar	Momento	$\sigma_c$	$\sigma_f$	As	hc,ef	Eps%	Sr,max	wd	Ve
30.	30.	3.	14.4892	-.81	29.51	8.04	7.5	.0084	13.82	.012	SI
152.	152.	3.	73.0561	-4.09	148.79	8.04	7.5	.0562	13.82	.078	SI

289.	289.	3.	1.	7.2553!	-.41	14.78	8.04	7.5	.0042	13.82	.006	SI
TENSIONI DI ESERCIZIO E FESSURAZIONE - FREQUENTI:												
Progressive	Se	Ar	Momento	$\sigma_c$	$\sigma_f$	As	hc,ef	Eps%	Sr,max	wd	Ve	
30.	30.	3.	1.	13.2824	-.74	27.05	8.04	7.5	.0077	13.82	.011	SI
152.	152.	3.	1.	67.0332!	-3.76!	136.52!	8.04	7.5	.0504	13.82	.07	SI
289.	289.	3.	1.	6.6433!	-.37!	13.53!	8.04	7.5	.0039	13.82	.005	SI
TENSIONI DI ESERCIZIO E FESSURAZIONE - QUASI PERMANENTI:												
Progressive	Se	Ar	Momento	$\sigma_c$	$\sigma_f$	As	hc,ef	Eps%	Sr,max	wd	Ve	
30.	30.	3.	1.	12.0858	-.68	24.61	8.04	7.5	.007	13.82	.01	SI
152.	152.	3.	1.	61.0104!	-3.42!	124.26!	8.04	7.5	.0446	13.82	.062	SI
289.	289.	3.	1.	6.0429!	-.34!	12.31!	8.04	7.5	.0035	13.82	.005	SI

## SELLA TIPO: III intermedia

DESCRIZIONE CAMPATE												
Cam.	Descriz.	S.ini	Sez.	S.fin	Incl.	L.assi	L.net.	lambda	K	r.Ar.	lam.max	
1	A201-A280	2	2	2	0	304.	304.	4.349	1.	1.575	28.727	
VERIFICHE ALLO STATO LIMITE ULTIMO												

FLESSIONE:													
Progressive	Se	Ar	Msd	Epsc	Epsc	Mrd	Epsc	Epsc	Cam	x/d	Mr/Ms	Ve	
> 15.	15.	3.	1.	-.0049	0.	0.	-99.2562!	-.046	.186	2.	.197	20310	SI
15.	15.	3.	1.	36.0329	-.014	.035	190.9458!	-.082	.186	2.	.306	5.299	SI
152.	152.	3.	1.	121.2534!	-.05!	.118!	190.9458!	-.082	.186	2.	.306	1.575	SI
274.	274.	3.	1.	-.0049	0.	0.	-99.2562!	-.046	.186	2.	.197	20310	SI

TAGLIO:												
Progressive	Se	Vsd	VRd	VRcd	VRsd	Asw	s	ctgT	Ve			
> 0.	0.	3.	159.33!	56.25	400.65	407.7	1.57	20.	2.2	SI		
70.	70.	3.	86.05!	71.07!	400.65	407.7	1.57	20.	2.2	SI		
304.	304.	3.	-159.33!	56.25	400.65	407.7	1.57	20.	2.2	SI		

VERIFICHE ALLO STATO LIMITE DI ESERCIZIO												
TENSIONI DI ESERCIZIO E FESSURAZIONE - RARE:												
Progressive	Se	Ar	Momento	$\sigma_c$	$\sigma_f$	As	hc,ef	Eps%	Sr,max	wd	Ve	
30.	30.	3.	1.	16.147	-.9	32.89	8.04	7.5	.0094	13.82	.013	SI
152.	152.	3.	1.	81.5113!	-4.57!	166.01!	8.04	7.5	.0644	13.82	.089	SI
289.	289.	3.	1.	8.0735!	-.45!	16.44!	8.04	7.5	.0047	13.82	.006	SI

TENSIONI DI ESERCIZIO E FESSURAZIONE - FREQUENTI:												
Progressive	Se	Ar	Momento	$\sigma_c$	$\sigma_f$	As	hc,ef	Eps%	Sr,max	wd	Ve	
30.	30.	3.	1.	14.8047	-.83	30.15	8.04	7.5	.0086	13.82	.012	SI
152.	152.	3.	1.	74.7356!	-4.19!	152.21!	8.04	7.5	.0579	13.82	.08	SI
289.	289.	3.	1.	7.4024!	-.41!	15.08!	8.04	7.5	.0043	13.82	.006	SI

TENSIONI DI ESERCIZIO E FESSURAZIONE - QUASI PERMANENTI:												
Progressive	Se	Ar	Momento	$\sigma_c$	$\sigma_f$	As	hc,ef	Eps%	Sr,max	wd	Ve	
30.	30.	3.	1.	13.4625	-.75	27.42	8.04	7.5	.0078	13.82	.011	SI
152.	152.	3.	1.	67.9598!	-3.81!	138.41!	8.04	7.5	.0513	13.82	.071	SI
289.	289.	3.	1.	6.7312!	-.38!	13.71!	8.04	7.5	.0039	13.82	.005	SI

## VERIFICA SEZIONI SELLE DI TESTA

### PARAMETRI GENERALI

Tipo verifica : stati limite - pressoflessione deviata.  
 Unità di misura generiche: kN; cm; kNm; N/mm<sup>2</sup>; d in mm; deformazioni\*1000.  
 ferri : diametri in mm; aree in cm<sup>2</sup>.  
 Simboli : Vert. = contorno\_vertice del CLS; d = diametro;  
 S = Sigma (tensioni sui materiali);  
 D = Deformazioni x 1000 (epsilon);  
 Ve = colonna che indica se la verifica e' soddisfatta;

### CARATTERISTICHE MATERIALI

Calcestruzzo: Rck = 30. ; fck = 24.9 ; fcd = 14.11 (.35%)  
 Acciaio : Tipo= B450C ; ftk = 540. ; fyk = 450. ; ftd = 469.565 (6.75%)

### CARATTERISTICHE SEZIONE

L'asse Z e' rivolto verso destra, l'asse Y e' rivolto verso l'alto.

Tipo sezione: Rettangolare 40x60 cm

Cls:		Acciaio lento:		ferro		Z	Y	d[mm]	Af[cm <sup>2</sup> ]
vert.	Z	Y		Z	Y				
1- 1	-20.	60.	1	17.	57.	20.		3.1416	
1- 2	20.	60.	2	10.2	57.	20.		3.1416	
1- 3	20.	0.	3	3.4	57.	20.		3.1416	
1- 4	-20.	0.	4	-3.4	57.	20.		3.1416	
			5	-10.2	57.	20.		3.1416	
			6	-17.	57.	20.		3.1416	
			7	17.	3.	16.		2.0106	
			8	-17.	3.	16.		2.0106	

### CASI DI CARICO

Nome	Descrizione	Tipo	Ses
1	SLU	SLU (statico)	1
2	SLU VENTOY	SLU (statico)	2
6	SLU con SISMAX PRINC	SLU (sismico)	32
7	SLU con SISMAX PRINC	SLU (sismico)	32
8	SLU con SISMAZ PRINC	SLU (sismico)	32
13	SLU Solo Perm.	SLU (statico)	1
16	Rara	RARA	1
17	Rara VentoY	RARA	2
18	Frequente	FREQUENTE	1
19	Frequente VentoY	FREQUENTE	2
20	Quasi Perm	QUASI PERMAN.	1

Sforzi normali applicati in z= 0. ; y= 0. (baricentro CLS)

Convenzioni: N + trazione; Mz + fib.inferiori tese; My + fib.sinistra tese.

## Descrizione : sella tipo

### SOLLECITAZIONI AGENTI

N.	N	Mz	My	Descrizione
1	191.91	-0.0763	-69.1033	Caso 2.1-A115- P1
2	195.26	28.6665	-0.0559	Caso 2.2-A115- P2
3	195.26	-0.0763	-69.1916	Caso 2.2-A115- P3
4	191.91	0.0763	69.1682	Caso 2.1-A115- P3
5	125.06	-0.0028	-2.5533	Caso 8.13-A115- P1
6	195.26	0.0763	69.0799	Caso 2.2-A115- P1

### RISULTATI

Piani di equilibrio (eps= muz \* y + muy \* z + lam):

Sol.	muz	muy	lambda
1.	-0.0003013603	0.00008009609	0.00226390093
2.	-0.0003251564	0.00000007545	0.00197154803
3.	-0.0003089268	0.00008083842	0.0023114519
4.	-0.0003030558	-0.00008025277	0.00227344818
5.	-0.0001132505	0.0000344639	0.00081140743
6.	-0.0003097876	-0.00008076748	0.00231521112

### DEFORMAZIONI SUI MATERIALI

Cls		Acciaio lento		ferri		S ferri		Ve	
sol	vert.	D cls	S cls	ferro	D ferri	S ferri	Ve		
1	1- 1	-1.1462	-11.54	si	1	1.9078	381.56	si	
1	1- 2	2.0577	0.	si	2	1.3631	272.63	si	
1	1- 3	3.8658	0.	si	3	.8185	163.69	si	
1	1- 4	.662	0.	si	4	.2738	54.76	si	
1					5	-.2708	-54.17	si	
1					6	-.8155	-163.1	si	
1					7	3.5351	393.19	si	
1					8	.8119	162.37	si	
2	1- 1	.0191	0.	si	1	.1194	23.89	si	
2	1- 2	.0221	0.	si	2	.1189	23.79	si	
2	1- 3	1.9731	0.	si	3	.1184	23.68	si	
2	1- 4	1.97	0.	si	4	.1179	23.58	si	
2					5	.1174	23.48	si	
2					6	.1169	23.37	si	
2					7	1.8753	375.06	si	
2					8	1.8727	374.54	si	
3	1- 1	-1.1589	-11.61	si	1	1.9248	384.96	si	
3	1- 2	2.0747	0.	si	2	1.3751	275.02	si	
3	1- 3	3.9282	0.	si	3	.8254	165.08	si	
3	1- 4	.6947	0.	si	4	.2757	55.14	si	
3					5	-.274	-54.8	si	
3					6	-.8237	-164.74	si	
3					7	3.593	393.26	si	
3					8	.8445	168.9	si	
4	1- 1	2.0602	0.	si	1	-.8183	-163.65	si	
4	1- 2	-1.1499	-11.56	si	2	-.2725	-54.51	si	
4	1- 3	.6684	0.	si	3	.2732	54.63	si	
4	1- 4	3.8785	0.	si	4	.8189	163.78	si	
4					5	1.3646	272.92	si	
4					6	1.9103	382.07	si	
4					7	.8182	163.65	si	
4					8	3.5468	393.2	si	

5	1- 1	.063	0.	si	1	.2245	44.89	si
5	1- 2	.2008	0.	si	2	.201	40.21	si
5	1- 3	.8803	0.	si	3	.1776	35.52	si
5	1- 4	.7425	0.	si	4	.1542	30.83	si
5					5	.1307	26.15	si
5					6	.1073	21.46	si
5					7	.836	167.2	si
5					8	.7188	143.77	si
6	1- 1	2.0718	0.	si	1	-.8236	-164.73	si
6	1- 2	-1.1589	-11.61	si	2	-.2744	-54.88	si
6	1- 3	.6999	0.	si	3	.2748	54.96	si
6	1- 4	3.9306	0.	si	4	.824	164.81	si
6					5	1.3733	274.65	si
6					6	1.9225	384.49	si
6					7	.8492	169.85	si
6					8	3.5953	393.26	si



## VERIFICA SEZIONI SELLE INTERMEDIE:

### PARAMETRI GENERALI

Tipo verifica : stati limite - pressoflessione deviata.  
 Unità di misura generiche: kN; cm; kNm; N/mm<sup>2</sup>; d in mm; deformazioni\*1000.  
 ferri : diametri in mm; aree in cm<sup>2</sup>.  
 Simboli : Vert. = contorno\_vertice del CLS; d = diametro;  
 S = Sigma (tensioni sui materiali);  
 D = Deformazioni x 1000 (epsilon);  
 Ve = colonna che indica se la verifica e' soddisfatta;

### CARATTERISTICHE MATERIALI

Calcestruzzo: Rck = 30. ; fck = 24.9 ; fcd = 14.11 (.35%)  
 Acciaio : Tipo= B450C ; ftk = 540. ; fyk = 450. ; ftd = 469.565 (6.75%)

### CARATTERISTICHE SEZIONE

L'asse Z e' rivolto verso destra, l'asse Y e' rivolto verso l'alto.

Tipo sezione: Rettangolare 25X70 cm

Cls:		Acciaio lento:		ferro		Z	Y	d[mm]	Af[cm <sup>2</sup> ]
vert.	Z	Y		Z	Y				
1- 1	-12.5	70.	1	9.5	67.	16.		2.0106	
1- 2	12.5	70.	2	-9.5	67.	16.		2.0106	
1- 3	12.5	0.	3	9.5	3.	16.		2.0106	
1- 4	-12.5	0.	4	3.2	3.	16.		2.0106	
			5	-3.2	3.	16.		2.0106	
			6	-9.5	3.	16.		2.0106	

### CASI DI CARICO

Nome	Descrizione	Tipo	Ses
1	SLU	SLU (statico)	1
2	SLU VENTOY	SLU (statico)	2
6	SLU con SISMAY PRINC	SLU (sismico)	32
7	SLU con SISMAY PRINC	SLU (sismico)	32
8	SLU con SISMAY PRINC	SLU (sismico)	32
13	SLU Solo Perm.	SLU (statico)	1
16	Rara	RARA	1
17	Rara VentoY	RARA	2
18	Frequente	FREQUENTE	1
19	Frequente VentoY	FREQUENTE	2
20	Quasi Perm	QUASI PERMAN.	1

Sforzi normali applicati in z= 0. ; y= 0. (baricentro CLS)  
 Convenzioni: N + trazione; Mz + fib.inferiori tese; My + fib.sinistra tese.

## Descrizione : Sella I intermedia

### SOLLECITAZIONI AGENTI

N.	N	Mz	My	Descrizione
1	134.84	-0.0418	-42.5815	Caso 2.1-A236- P1
2	157.92	83.207	-0.1189	Caso 2.2-A236- P2
3	157.92	-0.0381	-42.7971	Caso 2.2-A236- P3
4	134.84	0.0379	42.775	Caso 2.1-A236- P3
5	89.99	-0.0278	-28.3816	Caso 17.1-A236- P1

### RISULTATI

Piani di equilibrio (eps= muz \* y + muy \* z + lam):

Sol.	muz	muy	lambda
1.	.00007601274	.00299878269	.02685262155
2.	-.00002280443	.00000036471	.00136891946
3.	.00010880803	.00407612817	.03693567884
4.	.000077156	-.0030818732	.02762721502
5.	.00000733055	.00011788803	.00070241386

### DEFORMAZIONI SUI MATERIALI

sol	Cls		Acciaio lento		ferro		S ferri		Ve	
	vert.	D cls	S cls	Ve	D ferri	S ferri	Ve			
1	1- 1	-5.3113	-14.11	si	1	60.4339	361.13	si		
1	1- 2	69.6583	0.	si	2	3.457	393.1	si		
1	1- 3	64.3374	0.	si	3	55.5691	355.32	si		
1	1- 4	-10.6322	-14.11	si	4	36.5778	332.64	si		
1					5	17.5835	309.96	si		
1					6	-1.4078	-281.56	si		
2	1- 1	-.2319	-3.08	si	1	-.1555	-31.1	si		
2	1- 2	-.2228	-2.97	si	2	-.1624	-32.49	si		
2	1- 3	1.3735	0.	si	3	1.304	260.79	si		
2	1- 4	1.3644	0.	si	4	1.3017	260.33	si		
2					5	1.2994	259.87	si		
2					6	1.297	259.41	si		
3	1- 1	-6.3994	-14.11	si	1	82.949	388.01	si		
3	1- 2	95.5038	0.	si	2	5.5026	395.54	si		
3	1- 3	87.8873	0.	si	3	75.9853	379.7	si		
3	1- 4	-14.0159	-14.11	si	4	50.1712	348.87	si		
3					5	24.353	318.05	si		
3					6	-1.4611	-292.22	si		
4	1- 1	71.5515	0.	si	1	3.5189	393.17	si		
4	1- 2	-5.4953	-14.11	si	2	62.0745	363.09	si		
4	1- 3	-10.8962	-14.11	si	3	-1.4191	-283.82	si		
4	1- 4	66.1506	0.	si	4	18.0984	310.58	si		
4					5	37.619	333.89	si		
4					6	57.1365	357.19	si		
5	1- 1	-.258	-3.41	si	1	2.3135	391.73	si		
5	1- 2	2.6892	0.	si	2	.0736	14.72	si		
5	1- 3	2.176	0.	si	3	1.8443	368.87	si		
5	1- 4	-.7712	-8.78	si	4	1.0978	219.55	si		
5					5	.3511	70.21	si		
5					6	-.3955	-79.11	si		

## Descrizione : Sella II intermedia

### SOLLECITAZIONI AGENTI

N.	N	Mz	My	Descrizione
1	163.75	108.5707	-0.0057	Caso 2.2-A237- P2
2	129.47	-0.0322	-32.6417	Caso 2.1-A237- P1
3	86.3	-0.0215	-21.7615	Caso 17.1-A237- P1
4	163.75	0.0322	32.6247	Caso 2.2-A237- P1

**RISULTATI**

Piani di equilibrio (eps= muz * y +muy * z + lam):			
Sol.	muz	muy	lambda
1.	-0.00002855209	0.00000001607	0.00165633945
2.	0.00002152947	0.00028548992	0.00188340334
3.	0.00000465449	0.00008758387	0.00056173077
4.	0.00003330239	-0.00048855895	0.00356250444

**DEFORMAZIONI SUI MATERIALI**

sol	C/s			Ve	Acciaio lento			S ferri	Ve
	vert.	D c/s	S c/s		ferro	D ferri	S ferri		
1	1- 1	-0.3425	-4.42	si	1	-0.2565	-51.3	si	
1	1- 2	-0.3421	-4.41	si	2	-0.2568	-51.36	si	
1	1- 3	1.6565	0.	si	3	1.5708	314.17	si	
1	1- 4	1.6561	0.	si	4	1.5707	314.15	si	
1					5	1.5706	314.13	si	
1					6	1.5705	314.11	si	
2	1- 1	-0.1782	-2.4	si	1	6.038	396.18	si	
2	1- 2	6.9591	0.	si	2	0.6137	122.74	si	
2	1- 3	5.452	0.	si	3	4.6601	394.53	si	
2	1- 4	-1.6852	-13.76	si	4	2.8521	392.37	si	
2					5	1.0438	208.77	si	
2					6	-0.7642	-152.83	si	
3	1- 1	-0.2073	-2.77	si	1	1.7056	341.13	si	
3	1- 2	1.9823	0.	si	2	0.0415	8.31	si	
3	1- 3	1.6565	0.	si	3	1.4077	281.55	si	
3	1- 4	-0.5331	-6.52	si	4	0.8531	170.61	si	
3					5	0.2983	59.66	si	
3					6	-0.2564	-51.27	si	
4	1- 1	12.0007	0.	si	1	1.1525	230.49	si	
4	1- 2	-0.2133	-2.85	si	2	1.4351	201.43	si	
4	1- 3	-2.5445	-14.11	si	3	-0.9789	-195.78	si	
4	1- 4	9.6695	0.	si	4	2.1151	391.49	si	
4					5	5.2097	395.19	si	
4					6	8.3037	398.88	si	

**Descrizione : Sella III intermedia**

**SOLLECITAZIONI AGENTI**

N.	N	Mz	My	Descrizione
1	163.41	121.2534	-0.0097	Caso 2.2-A238- P2
2	97.73	-0.0049	-4.9797	Caso 6.14-A238- P3
3	97.73	0.0049	4.9689	Caso 6.14-A238- P1
4	86.53	-0.0001	-0.0598	Caso 17.1-A238- P1
5	163.41	0.0001	0.0702	Caso 2.2-A238- P1

**RISULTATI**

Piani di equilibrio (eps= muz * y +muy * z + lam):			
Sol.	muz	muy	lambda
1.	-0.00003132382	0.00000002656	0.00179092655
2.	0.00000462936	0.00003144821	0.00029808504
3.	0.00000462677	-0.00003139554	0.00029804988
4.	0.00000420283	0.00000039025	0.00025636862
5.	0.00000793682	-0.00000045813	0.00048414969

**DEFORMAZIONI SUI MATERIALI**

sol	C/s			Ve	Acciaio lento			S ferri	Ve
	vert.	D c/s	S c/s		ferro	D ferri	S ferri		
1	1- 1	-0.4021	-5.1	si	1	-0.3075	-61.5	si	
1	1- 2	-0.4014	-5.1	si	2	-0.308	-61.6	si	
1	1- 3	1.7913	0.	si	3	1.6972	339.44	si	
1	1- 4	1.7906	0.	si	4	1.697	339.41	si	
1					5	1.6969	339.37	si	
1					6	1.6967	339.34	si	
2	1- 1	0.229	0.	si	1	0.907	181.4	si	
2	1- 2	1.0152	0.	si	2	0.3095	61.9	si	
2	1- 3	0.6912	0.	si	3	0.6107	122.15	si	
2	1- 4	-0.095	-1.31	si	4	0.4116	82.31	si	
2					5	0.2124	42.48	si	
2					6	0.0132	2.64	si	
3	1- 1	1.0144	0.	si	1	0.3098	61.96	si	
3	1- 2	0.2295	0.	si	2	0.9063	181.26	si	
3	1- 3	-0.0944	-1.3	si	3	0.0137	2.73	si	
3	1- 4	0.6905	0.	si	4	0.2125	42.5	si	
3					5	0.4114	82.27	si	
3					6	0.6102	122.04	si	
4	1- 1	0.5457	0.	si	1	0.5417	108.33	si	
4	1- 2	0.5554	0.	si	2	0.5343	106.85	si	
4	1- 3	0.2612	0.	si	3	0.2727	54.54	si	
4	1- 4	0.2515	0.	si	4	0.2702	54.04	si	
4					5	0.2677	53.55	si	
4					6	0.2653	53.05	si	
5	1- 1	1.0455	0.	si	1	1.0116	202.31	si	
5	1- 2	1.034	0.	si	2	1.0203	204.05	si	
5	1- 3	0.4784	0.	si	3	0.5036	100.72	si	
5	1- 4	0.4899	0.	si	4	0.5065	101.3	si	
5					5	0.5094	101.88	si	
5					6	0.5123	102.46	si	

## VERIFICA CATENE IN ACCIAIO

### PARAMETRI GENERALI

Tipo verifica : stati limite.  
 Unità di misura generiche: kN; cm; kNm; N/mm<sup>2</sup>.

### CARATTERISTICHE MATERIALI

S235 (EN 10025-2): Mod.EI.= 210000.;  $\rho_m = 1.050$ ;  
 $f_{yk} = 235$ .( 215. per  $sp > 40$  mm);  $f_{yd} = 224$ .( 205. per  $sp > 40$  mm).

### CARATTERISTICHE SEZIONE

L'asse Z e' rivolto verso destra, l'asse Y e' rivolto verso l'alto.

Tipo sezione: CIRCOLARE 2.0 cm

A = 3.14159E+00 Jz= 0.7854E+00 Jy= 0.7854E+00 Jt= 1.5708E+00

### CASI DI CARICO

Nome	Descrizione	Tipo	Ses
1	SLU	SLU (statico)	1
2	SLU VENTOY	SLU (statico)	2
6	SLU con SISMAX PRINC	SLU (sismico)	32
7	SLU con SISMAX PRINC	SLU (sismico)	32
8	SLU con SISMAX PRINC	SLU (sismico)	32
13	SLU Solo Perm.	SLU (statico)	1
16	Rara	RARA	1
17	Rara VentoY	RARA	2
18	Frequente	FREQUENTE	1
19	Frequente VentoY	FREQUENTE	2
20	Quasi Perm	QUASI PERMAN.	1

sforzi normali applicati in z= 0. ; y= 0. (baricentro CLS)

Convenzioni: N + trazione; Mz + fib.inferiori tese; My + fib.sinistra tese.

## Descrizione : Catena tipo

SEZIONE CIRCOLARE stato limite ultimo - ASTE da A2001 a A2224

PROGR. 0.

### SOLLECITAZIONI :

Caso	MZ	MY	MT	N	TZ	TY
13- 1	0.00000	0.	0.	-73.782	0.000	0.040
1- 1	0.00000	0.	0.	-73.355	0.000	0.040

TENSIONI (Sz= 0.0) :

Caso	Ve	No	massimi	Sx	Tz	Ty	Si
13- 1	si	1	Sx	-106.	0.	0.	106.
1- 1	si	5	Ty	-106.	0.	0.	106.
13- 1	si	5	Si	-106.	0.	0.	106.

PROGR. 152.

### SOLLECITAZIONI :

Caso	MZ	MY	MT	N	TZ	TY
2- 2	0.03008	0.	0.	1.670	0.000	0.000
2- 1	0.03008	0.	0.	1.619	0.000	0.000

TENSIONI (Sz= 0.0) :

Caso	Ve	No	massimi	Sx	Tz	Ty	Si
2- 2	si	9	Sx	14.	0.	0.	14.
2- 1	si	5	Ty	2.	0.	0.	2.

PROGR. 304.

### SOLLECITAZIONI :

Caso	MZ	MY	MT	N	TZ	TY
2- 2	0.00000	0.	0.	1.670	0.000	-0.040
1- 1	0.00000	0.	0.	1.645	0.000	-0.040

TENSIONI (Sz= 0.0) :

Caso	Ve	No	massimi	Sx	Tz	Ty	Si
2- 2	si	1	Sx	2.	0.	0.	2.
1- 1	si	5	Ty	2.	0.	0.	2.
2- 2	si	5	Si	2.	0.	0.	2.

### VERIFICA STABILITA` :

L0 = 304.  
 Z | Lc = 0. | Ro = 0.74 | lm = 0.0 | Ncr= Infinity | alfa(c )=0.4900 | ki=1.0000 |  
 Y | Lc = 0. | Ro = 0.74 | lm = 0.0 | Ncr= Infinity | alfa(c )=0.4900 | ki=1.0000 |

Caso13- 1 - Nodo 1 - Asse Z  
 Ned = -73.782 | Mzeq = 0.02607 | Myeq = 0.00000 | Ss = -116. ( 0.520)

### Rapporti di tensioni:

asta	sez	profilo	Tau %	Sx %	Si %	Ss %	Max %
	7	CIRCOLARE 2.0 cm	1	48	48	52	Ss

## VERIFICA TRAVI NORD

### PARAMETRI GENERALI

Metodo di verifica : stati limite - NTC18 ->  
 Duttilita' : non prevista (struttura non dissipativa)  
 Unita' di misura : cm; kN; kN/m; kNm; N/mm2; deform. %.  
 Unita' particolari : fessure [wk]:mm - ferri:mm e cm2 - sezioni:cm e derivate.  
 Copriferri (assi) : longitudinali= 3 ; staffe= 2

### CARATTERISTICHE MATERIALI

CLS : Rck =30. ; fck=24.9 ; fctk= 1.79; fctm= 2.56; Ec= 31447.2 ;  
 gc =1.5 ; fcd=14.11; fbd= 2.69; fctd= 1.19; Ecu=.2% (limit.elastico)  
 ACCIAIO : B450C; ftk=517.5 ; fyk=450. ; Es=210000. ;  
 gs =1.15; fyd=391.3 ; ftd(k\*fyd)=450. ; fud=443.98; Eud=.19% (limit.elastico)  
 Fibre : Tipo = FRCM in PBO ; Samm= 1036.3 ; n=12.45 ; (valutate alle TA)

### VALORI TENSIONI MASSIME IN ESERCIZIO

GRUPPO : aggressivo.

CLS :  $\sigma_c$  (rara)=14.94;  $\sigma_c$  (quasi permanente)=11.2 ; fbd(esercizio)= 2.69  
 ACCIAIO :  $\sigma_f$  (rara)=360. ; Coeff.Omogeneizzazione= 15  
 FESSURE : wmax(fre.)=.3 ; wmax(q.p.)=.2 [4.1.2.2.4.5]; kt=.4 [EN 1992-1 7.3.4].

### CASI DI CARICO

Nome	Descrizione	Sest
1.	SLU	1.
2.	SLU VENTYOY	2.
6.	SLU con SISMAX PRINC32	
7.	SLU con SISMAX PRINC32	
8.	SLU con SISMAZ PRINC32	
13.	SLU Solo Perm.	1.

RARE			FREQUENTI			QUASI PERMANENTI		
Nome	Descrizione	Sest	Nome	Descrizione	Sest	Nome	Descrizione	Sest
16.	Rara	1.	18.	Frequente	1.	20.	Quasi Perm	1.
17.	Rara VentoY	2.	19.	Frequente VentoY	2.			

### SEZIONI UTILIZZATE

5) Sezione a Z (1): 90/20/57X190/25/25; A=6475.; Jg=29062264.; E=31447.16

### ARMATURE LONGITUDINALI (%=100\*Af/Acls - Acls=area intera sezione)

Nro	Totale	%	Super.	%	Barre			Infer.	%	Barre		
1	88.91	1.373	46.43	.717	2d20	+2d26	+2d2	...	42.47	.656	4d26	+4d26
2	91.92	1.42	47.44	.733	2d20	+2d26	+2d2	...	44.48	.687	4d26	+2d8 +2d8 ...
3	95.94	1.482	47.44	.733	2d20	+2d26	+2d2	...	48.51	.749	4d26	+2d8 +2d8 ...
4	95.94	1.482	36.82	.569	2d20	+2d26	+2d2	...	59.12	.913	4d26	+2d26 +2d8 ...
5	95.94	1.482	26.2	.405	2d20	+2d26	+1d8	...	69.74	1.077	4d26	+2d26 +2d2 ...
6	95.94	1.482	15.58	.241	2d20	+1d8	+1d20	...	80.36	1.241	4d26	+2d26 +2d2 ...
F	5.00	-	-	-	Long.Faccia esterna			-	-	Long.Faccia esterna		

## TRAVE TIPO:

DESCRIZIONE CAMPATE												
Cam.	Descriz.	S.ini	Sez.	S.fin	Incl.	L.assi	L.net.	lambda	K	r.Ar.	lam.max	
1	A3155	5	5	5	0	34.	34.	.179	.4	5.	29.848	
2	A3156	5	5	5	0	35.	20.	.184	1.5	1.	22.386	
3	A3157	5	5	5	0	35.	20.	.184	1.5	5.	107.842	
4	A3158	5	5	5	0	110.	110.	.579	1.5	4.398	94.861	
5	A3159	5	5	5	0	35.	35.	.184	1.5	3.861	83.274	
6	A3160	5	5	5	0	35.	35.	.184	1.5	3.357	72.4	
7	A3161	5	5	5	0	440.	440.	2.316	1.5	1.737	37.461	
8	A3162	5	5	5	0	35.	35.	.184	1.5	1.669	36.	
9	A3163	5	5	5	0	35.	35.	.184	1.5	1.633	35.214	
10	A3164	5	5	5	0	440.	440.	2.316	1.5	1.369	29.525	
11	A3165	5	5	5	0	35.	35.	.184	1.5	1.359	29.31	
12	A3166	5	5	5	0	35.	35.	.184	1.5	1.359	29.312	
13	A3167	5	5	5	0	440.	440.	2.316	1.5	1.369	29.533	
14	A3168	5	5	5	0	35.	35.	.184	1.5	1.642	35.409	
15	A3169	5	5	5	0	35.	35.	.184	1.5	1.679	36.204	
16	A3170	5	5	5	0	440.	440.	2.316	1.5	1.747	37.681	
17	A3171	5	5	5	0	35.	35.	.184	1.5	3.359	72.441	
18	A3172	5	5	5	0	35.	35.	.184	1.5	3.898	84.075	
19	A3173	5	5	5	0	110.	110.	.579	1.5	4.806	103.658	
20	A3174	5	5	5	0	35.	20.	.184	1.5	5.	107.842	
21	A3175	5	5	5	0	35.	20.	.184	1.5	1.	22.386	
22	A3176	5	5	5	0	34.	34.	.179	.4	1.	5.97	

### VERIFICHE ALLO STATO LIMITE ULTIMO

FLESSIONE:												
Progressive	SE	Ar	Msd	Epsc	Epsac	Mrd	Epsc	Epsac	Cam	x/d	Mr/Ms	VE
> 9.	9.	5.	1.	-1.2852	0.	0.	-3659.907	-.09	.186	2.	.325	2848. !SI
34.	34.	5.	1.	-1.2852	0.	0.	-3659.269	-.09	.186	2.	.325	2847. !SI
> 34.	0.	5.	1.	-4.2993	0.	0.	-3659.297	-.09	.186	2.	.325	851.1 !SI
43.	9.	5.	2.	-4.2993	0.	0.	-3860.921	-.093	.186	2.	.333	898. !SI
69.	35.	5.	2.	-4.2993	0.	0.	-3661.472	-.091	.186	2.	.327	851.6 !SI
> 69.	0.	5.	2.	-5.3947	0.	0.	-3661.472	-.091	.186	2.	.327	678.7 !SI
69.	0.	5.	2.	206.1238	-.004	.011	3457.5986	-.067	.186	2.	.265	16.77 !SI
78.	9.	5.	2.	206.1238	-.004	.011	3455.5429	-.068	.186	2.	.268	16.76 !SI
104.	35.	5.	2.	-5.3947	0.	0.	-3346.869	-.086	.186	2.	.315	620.4 !SI
104.	35.	5.	2.	206.1238	-.004	.011	3502.3727	-.071	.186	2.	.276	16.99 !SI
> 104.	0.	5.	2.	706.6649	-.013	.037	3502.3308	-.071	.186	2.	.276	4.956 !SI
128.	24.	5.	3.	846.1848	-.016	.043	3617.1128	-.074	.186	2.	.283	4.275 !SI
149.	45.	5.	3.	858.3149	-.016	.042	3774.9869	-.076	.186	2.	.29	4.398 !SI
214.	110.	5.	3.	858.3149	-.015	.039	4085.9194	-.079	.186	2.	.299	4.76 !SI
> 214.	0.	5.	3.	1058.268	-.019	.048	4085.8868	-.079	.186	2.	.299	3.861 !SI
249.	35.	5.	3.	1058.268	-.019	.048	4086.1124	-.079	.186	2.	.299	3.861 !SI
> 249.	0.	5.	3.	1217.2718	-.022	.055	4086.1127	-.079	.186	2.	.299	3.357 !SI
275.	26.	5.	3.	1217.2718	-.023	.056	4045.7439	-.082	.186	2.	.306	3.324 !SI
284.	35.	5.	3.	1217.2718	-.023	.056	4045.7317	-.083	.186	2.	.308	3.324 !SI
> 284.	0.	5.	3.	1574.7959	-.03	.072	4045.1655	-.083	.186	2.	.308	2.569 !SI
667.	383.	5.	5.	2979.5448	-.06	.106	5175.0759	-.113	.186	2.	.378	1.737 !SI
724.	440.	5.	5.	2979.5448	-.06	.106	5176.9997	-.113	.186	2.	.378	1.738 !SI

> 724.	0.	5.	5.	3101.5906	-.062	.111	5176.9436	-.113	.186	2.	.378	1.669	SI
759.	35.	5.	5.	3101.5906	-.062	.111	5177.2829	-.113	.186	2.	.378	1.669	SI
> 759.	0.	5.	5.	3171.0279	-.064	.113	5177.2833	-.113	.186	2.	.378	1.633	SI
794.	35.	5.	5.	3171.0279	-.064	.113	5177.2833	-.113	.186	2.	.378	1.633	SI
> 794.	0.	5.	5.	3313.9519	-.067	.118	5176.512	-.113	.186	2.	.378	1.562	SI
1177.	383.	5.	5.	3790.9672	-.078	.135	5189.5353	-.114	.186	2.	.379	1.369	SI
1234.	440.	5.	5.	3790.9672	-.078	.135	5191.4635	-.114	.186	2.	.379	1.369	SI
>1234.	0.	5.	5.	3820.2158	-.079	.136	5191.4072	-.114	.186	2.	.379	1.359	SI
1269.	35.	5.	5.	3820.2158	-.079	.136	5191.7476	-.114	.186	2.	.379	1.359	SI
>1269.	0.	5.	5.	3820.2159	-.079	.136	5191.7479	-.114	.186	2.	.379	1.359	SI
1304.	35.	5.	5.	3820.2159	-.079	.136	5191.7479	-.114	.186	2.	.379	1.359	SI
>1304.	0.	5.	5.	3790.9836	-.078	.135	5190.9742	-.114	.186	2.	.379	1.369	SI
1744.	440.	5.	5.	3313.9457	-.067	.118	5205.9716	-.114	.186	2.	.379	1.571	SI
>1744.	0.	5.	5.	3171.0162	-.064	.113	5205.9152	-.114	.186	2.	.379	1.642	SI
1779.	35.	5.	5.	3171.0162	-.064	.113	5206.2566	-.114	.186	2.	.379	1.642	SI
>1779.	0.	5.	5.	3101.5951	-.062	.11	5206.2569	-.114	.186	2.	.379	1.679	SI
1814.	35.	5.	5.	3101.5951	-.062	.11	5206.2569	-.114	.186	2.	.379	1.679	SI
>1814.	0.	5.	5.	2979.5576	-.059	.106	5205.4809	-.114	.186	2.	.379	1.747	SI
1936.	122.	5.	5.	2837.0725	-.056	.101	5209.6482	-.114	.186	2.	.379	1.836	SI
2254.	440.	5.	3.	1574.7842	-.03	.071	4088.3734	-.084	.186	2.	.31	2.596	SI
>2254.	0.	5.	3.	1217.254	-.023	.055	4088.3315	-.084	.186	2.	.31	3.359	SI
2263.	9.	5.	3.	1217.254	-.023	.055	4087.3752	-.083	.186	2.	.307	3.358	SI
2289.	35.	5.	3.	1217.254	-.022	.055	4125.229	-.08	.186	2.	.3	3.389	SI
>2289.	0.	5.	3.	1058.2767	-.019	.048	4125.2292	-.08	.186	2.	.3	3.898	SI
2324.	35.	5.	3.	1058.2767	-.019	.048	4125.2292	-.08	.186	2.	.3	3.898	SI
>2324.	0.	5.	3.	858.3399	-.015	.039	4125.1824	-.08	.186	2.	.3	4.806	SI
2348.	24.	5.	3.	858.3399	-.015	.039	4125.9728	-.08	.186	2.	.301	4.807	SI
2410.	86.	5.	3.	846.2167	-.016	.043	3660.4709	-.074	.186	2.	.285	4.326	SI
2434.	110.	5.	2.	706.6623	-.013	.037	3545.3879	-.072	.186	2.	.278	5.017	SI
>2434.	0.	5.	2.	-5.3947	0.	0.	-3314.031	-.084	.186	2.	.312	614.3	SI
2434.	0.	5.	2.	206.1076	-.004	.011	3545.3563	-.072	.186	2.	.278	17.2	SI
2460.	26.	5.	2.	206.1076	-.004	.011	3496.0264	-.069	.186	2.	.271	16.96	SI
2469.	35.	5.	2.	-5.3947	0.	0.	-3627.602	-.09	.186	2.	.325	672.4	SI
2469.	35.	5.	2.	206.1076	-.004	.011	3496.9754	-.068	.186	2.	.268	16.97	SI
>2469.	0.	5.	2.	-4.2993	0.	0.	-3627.602	-.09	.186	2.	.325	843.8	SI
2493.	24.	5.	2.	-4.2993	0.	0.	-3826.952	-.092	.186	2.	.331	890.1	SI
2504.	35.	5.	1.	-4.2993	0.	0.	-3629.632	-.089	.186	2.	.323	844.2	SI
>2504.	0.	5.	1.	-1.2852	0.	0.	-3629.645	-.089	.186	2.	.323	2824.	SI
2529.	25.	5.	1.	-1.2852	0.	0.	-3629.044	-.089	.186	2.	.323	2824.	SI

TAGLIO:

Progressive	Se	Vsd	VRd	VRcd	VRsd	Asw	s	ctgT	Ve							
> 0.	0.	5.	0.	99.85	921.79	926.89	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO	
34.	34.	5.	-7.56	99.85	921.79	926.89	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO	
> 34.	0.	5.	-7.6	99.85	921.79	926.89	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO	
69.	35.	5.	-12.05	99.85	921.79	926.89	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO	
> 69.	0.	5.	607.79	144.02	921.79	926.89	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO	
93.	24.	5.	602.36	181.45	921.79	926.89	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO	
104.	35.	5.	600.01	181.45	921.79	926.89	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO	
> 104.	0.	5.	599.9	181.45	921.79	926.89	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO	
128.	24.	5.	594.56	195.46	921.79	926.89	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO	
214.	110.	5.	575.22	195.46	921.79	926.89	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO	
> 214.	0.	5.	575.25	195.46	921.79	926.89	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO	
249.	35.	5.	567.47	195.46	921.79	926.89	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO	
> 249.	0.	5.	458.11	195.46	921.79	926.89	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO	
275.	26.	5.	452.42	216.29	921.79	926.89	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO	
284.	35.	5.	450.32	216.29	921.79	926.89	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO	
> 284.	0.	5.	450.27	216.29	921.79	926.89	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO	
341.	57.	5.	437.69	219.12	921.79	926.89	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO	
500.	226.	5.	437.69	219.12	921.79	926.89	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO	
724.	214.	5.	352.63	219.12	818.87	501.56	1.01	33.	2.5	SI						
> 724.	0.	5.	352.63	219.12	818.87	501.56	1.01	33.	2.5	SI						
759.	35.	5.	344.85	219.12	818.87	501.56	1.01	33.	2.5	SI						
> 759.	0.	5.	202.19	219.12	818.87	501.56	1.01	33.	2.5	SI						
794.	35.	5.	194.4	219.12	818.87	501.56	1.01	33.	2.5	SI						
> 794.	0.	5.	194.34	219.12	818.87	501.56	1.01	33.	2.5	SI						
1234.	440.	5.	87.51	219.12	818.87	501.56	1.01	33.	2.5	SI						
>1234.	0.	5.	87.49	219.12	818.87	501.56	1.01	33.	2.5	SI						
1269.	35.	5.	79.7	219.12	818.87	501.56	1.01	33.	2.5	SI						
>1269.	0.	5.	-79.66	219.12	818.87	501.56	1.01	33.	2.5	SI						
1304.	35.	5.	-87.44	219.12	818.87	501.56	1.01	33.	2.5	SI						
>1304.	0.	5.	-87.51	219.12	818.87	501.56	1.01	33.	2.5	SI						
1744.	440.	5.	-194.35	219.12	818.87	501.56	1.01	33.	2.5	SI						
>1744.	0.	5.	-194.36	219.12	818.87	501.56	1.01	33.	2.5	SI						
1779.	35.	5.	-202.14	219.12	818.87	501.56	1.01	33.	2.5	SI						
>1779.	0.	5.	-344.8	219.12	818.87	501.56	1.01	33.	2.5	SI						
1814.	35.	5.	-352.58	219.12	818.87	501.56	1.01	33.	2.5	SI						
>1814.	0.	5.	-352.66	219.12	818.87	501.56	1.01	33.	2.5	SI						
1814.	214.	5.	-352.66	219.12	818.87	501.56	1.01	33.	2.5	SI						
2028.	226.	5.	-450.28	216.29	921.79	926.89	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO	
>2254.	0.	5.	-450.28	216.29	921.79	926.89	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO	
2289.	35.	5.	-458.06	195.46	921.79	926.89	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO	
>2289.	0.	5.	-567.42	195.46	921.79	926.89	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO	
2324.	35.	5.	-575.2	195.46	921.79	926.89	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO	
>2324.	0.	5.	-575.25	195.46	921.79	926.89	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO	
2434.	110.	5.	-599.94	181.45	921.79	926.89	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO	
>2434.	0.	5.	-599.97	181.45	921.79	926.89	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO	
2469.	35.	5.	-607.75	99.85	921.79	926.89	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO	
>2469.	0.	5.	12.05	99.85	921.79	926.89	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO	
2504.	35.	5.	7.6	99.85	921.79	926.89	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO	
>2504.	0.	5.	7.56	99.85	921.79	926.89	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO	
2538.	34.	5.	0.	99.85	921.79	926.89	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO	
2538.	34.	5.	0.	99.85	921.79	926.89	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO	

VERIFICHE ALLO STATO LIMITE DI ESERCIZIO

TENSIONI DI ESERCIZIO E FESSURAZIONE - RARE:

Progressive	Se	Ar	Momento	σc	σf	As	hc,ef	Eps%	Sr,max	wd	Ve	
> 9.	9.	5.	1.	-1.368	0.	.01	41.28	7.54	0.	12.63	0.	SI

9.	9.	5.	1.	- .1368	0.	.01	41.28	7.54	0.	12.63	0.	SI
34.	34.	5.	1.	- .9842	0.	.1	41.28	7.67	0.	12.92	0.	SI
> 34.	0.	5.	1.	- .9842	0.	.1	41.28	7.67	0.	12.91	0.	SI
54.	20.	5.	2.	-2.5205	- .01	.26	41.28	8.6	.0001	13.78	0.	SI
60.	26.	5.	2.	-2.986	- .01	.31	56.99	23.86	.0001	20.74	0.	SI
69.	35.	5.	2.	-3.2923	- .01	.35	56.99	30.1	.0001	24.6	0.	SI
> 69.	0.	5.	2.	-4.1225	- .01	.44	56.99	30.1	.0001	24.6	0.	SI
78.	9.	5.	2.	36.8513	- .1	4.16	45.62	11.96	.0012	11.98	.001	SI
84.	15.	5.	2.	60.8466	- .16	6.85	45.62	11.96	.002	11.98	.002	SI
104.	35.	5.	2.	146.7961	- .39	16.32	45.62	11.96	.0047	11.98	.006	SI
> 104.	0.	5.	2.	146.7961	- .39	16.32	45.62	11.96	.0047	11.99	.006	SI
214.	110.	5.	3.	611.2145	-1.54	58.24	56.23	14.38	.0174	11.2	.019	SI
> 214.	0.	5.	3.	611.2145	-1.54	58.24	56.23	14.38	.0174	11.21	.019	SI
249.	35.	5.	3.	753.2939	-1.89	71.78	56.23	14.35	.0238	11.15	.027	SI
> 249.	0.	5.	3.	753.3161	-1.89	71.78	56.23	14.35	.0238	11.15	.027	SI
284.	35.	5.	3.	867.493	-2.28	83.27	56.23	14.35	.0293	11.15	.033	SI
> 284.	0.	5.	3.	867.493	-2.28	83.28	56.23	14.42	.0293	11.28	.033	SI
724.	440.	5.	5.	2122.1995	-5.54	157.95	77.47	17.86	.0656	8.83	.058	SI
> 724.	0.	5.	5.	2122.1995	-5.54	157.95	77.47	17.87	.0656	8.84	.058	SI
759.	35.	5.	5.	2208.101	-5.76	164.33	77.47	17.84	.0686	8.79	.06	SI
> 759.	0.	5.	5.	2208.1237	-5.76	164.33	77.47	17.84	.0686	8.79	.06	SI
794.	35.	5.	5.	2258.3497	-5.89	168.07	77.47	17.84	.0704	8.79	.062	SI
> 794.	0.	5.	5.	2258.3497	-5.89	168.1	77.47	17.9	.0704	8.91	.063	SI
851.	57.	5.	5.	2327.3249	-6.07	173.16	77.47	17.73	.0728	8.62	.063	SI
1234.	440.	5.	5.	2698.3381	-7.02	200.23	77.47	16.56	.0862	6.65	.057	SI
>1234.	0.	5.	5.	2698.3381	-7.02	200.24	77.47	16.57	.0862	6.66	.057	SI
1269.	35.	5.	5.	2718.1279	-7.07	201.69	77.47	16.54	.0869	6.6	.057	SI
>1269.	0.	5.	5.	2718.128	-7.07	201.69	77.47	16.54	.0869	6.6	.057	SI
1304.	35.	5.	5.	2698.3498	-7.02	200.22	77.47	16.54	.0862	6.6	.057	SI
>1304.	0.	5.	5.	2698.3498	-7.02	200.25	77.47	16.6	.0862	6.72	.058	SI
1744.	440.	5.	5.	2258.3413	-5.86	167.09	77.47	15.26	.0708	5.48	.039	SI
>1744.	0.	5.	5.	2258.3413	-5.86	167.09	77.47	15.27	.0708	5.49	.039	SI
1779.	35.	5.	5.	2208.1269	-5.73	163.36	77.47	15.24	.0691	5.44	.038	SI
>1779.	0.	5.	5.	2208.1042	-5.73	163.36	77.47	15.24	.0691	5.44	.038	SI
1814.	35.	5.	5.	2122.2088	-5.51	157.01	77.47	15.24	.0661	5.44	.036	SI
>1814.	0.	5.	5.	2122.2088	-5.51	157.03	77.47	15.3	.066	5.56	.037	SI
2001.	187.	5.	5.	1622.9036	-4.35	129.16	66.85	11.93	.0533	14.63	.078	SI
2254.	440.	5.	3.	867.4803	-2.27	82.34	56.23	9.18	.0314	14.24	.045	SI
>2254.	0.	5.	3.	867.4803	-2.27	82.34	56.23	9.18	.0314	14.25	.045	SI
2289.	35.	5.	3.	753.3223	-1.89	71.05	56.23	9.15	.0261	14.23	.037	SI
>2289.	0.	5.	3.	753.3001	-1.89	71.05	56.23	9.15	.0261	14.23	.037	SI
2324.	35.	5.	3.	611.2324	-1.53	57.65	56.23	9.15	.0197	14.23	.028	SI
>2324.	0.	5.	3.	611.2324	-1.53	57.65	56.23	9.16	.0197	14.24	.028	SI
2434.	110.	5.	2.	146.7844	- .39	16.11	45.62	5.78	.0046	13.33	.006	SI
>2434.	0.	5.	2.	146.7844	- .39	16.11	45.62	5.79	.0046	13.33	.006	SI
2469.	35.	5.	2.	-4.1225	- .01	.44	56.99	36.29	.0001	32.57	0.	SI
>2469.	0.	5.	2.	-3.2923	- .01	.35	56.99	36.29	.0001	32.57	0.	SI
2504.	35.	5.	1.	- .9842	0.	.11	56.99	25.75	0.	26.08	0.	SI
>2504.	0.	5.	1.	- .9842	0.	.11	56.99	25.75	0.	26.07	0.	SI
2529.	25.	5.	1.	- .1368	0.	.01	56.99	25.88	0.	26.15	0.	SI

TENSIONI DI ESERCIZIO E FESSURAZIONE - FREQUENTI:

Progressive	Se	Ar	Momento	$\sigma_c$	$\sigma_f$	As	hc,ef	Eps%	Sr,max	wd	Vel	
> 9.	9.	5.	1.	- .1328	0.	.01	41.28	7.54	0.	12.63	0.	SI
9.	9.	5.	1.	- .1328	0.	.01	41.28	7.54	0.	12.63	0.	SI
> 34.	34.	5.	1.	- .9553	0.	.1	41.28	7.67	0.	12.92	0.	SI
34.	34.	5.	1.	- .9553	0.	.1	41.28	7.67	0.	12.91	0.	SI
54.	20.	5.	2.	-2.4467	- .01	.25	41.28	8.6	.0001	13.78	0.	SI
60.	26.	5.	2.	-2.8985	- .01	.3	56.99	23.86	.0001	20.74	0.	SI
69.	35.	5.	2.	-3.1959	- .01	.34	56.99	30.1	.0001	24.6	0.	SI
> 69.	0.	5.	2.	-3.9567	- .01	.42	56.99	30.1	.0001	24.6	0.	SI
78.	9.	5.	2.	34.0642	- .09	3.84	45.62	11.96	.0011	11.98	.001	SI
84.	15.	5.	2.	56.3802	- .15	6.35	45.62	11.96	.0018	11.98	.002	SI
104.	35.	5.	2.	136.2934	- .36	15.15	45.62	11.96	.0043	11.98	.005	SI
> 104.	0.	5.	2.	136.2934	- .36	15.15	45.62	11.96	.0043	11.99	.005	SI
214.	110.	5.	3.	567.5621	-1.43	54.08	56.23	14.38	.0155	11.2	.017	SI
> 214.	0.	5.	3.	567.5621	-1.43	54.08	56.23	14.38	.0155	11.21	.017	SI
249.	35.	5.	3.	699.3221	-1.76	66.63	56.23	14.35	.0214	11.15	.024	SI
> 249.	0.	5.	3.	699.3265	-1.76	66.64	56.23	14.35	.0214	11.15	.024	SI
284.	35.	5.	3.	805.3232	-2.12	77.3	56.23	14.35	.0265	11.15	.03	SI
> 284.	0.	5.	3.	805.3232	-2.12	77.31	56.23	14.42	.0264	11.28	.03	SI
724.	440.	5.	5.	1962.6599	-5.12	146.07	77.47	17.86	.0599	8.83	.053	SI
> 724.	0.	5.	5.	1962.6599	-5.12	146.07	77.47	17.87	.0599	8.84	.053	SI
759.	35.	5.	5.	2041.2111	-5.32	151.91	77.47	17.84	.0627	8.79	.055	SI
> 759.	0.	5.	5.	2041.2157	-5.32	151.91	77.47	17.84	.0627	8.79	.055	SI
794.	35.	5.	5.	2086.9183	-5.44	155.31	77.47	17.84	.0643	8.79	.057	SI
> 794.	0.	5.	5.	2086.9183	-5.44	155.34	77.47	17.9	.0643	8.91	.057	SI
851.	57.	5.	5.	2148.9426	-5.6	159.89	77.47	17.73	.0665	8.62	.057	SI
1234.	440.	5.	5.	2488.5318	-6.47	184.66	77.47	16.56	.0787	6.65	.052	SI
>1234.	0.	5.	5.	2488.5318	-6.47	184.67	77.47	16.57	.0787	6.66	.052	SI
1269.	35.	5.	5.	2506.7349	-6.52	186.	77.47	16.54	.0794	6.6	.052	SI
>1269.	0.	5.	5.	2506.7349	-6.52	186.	77.47	16.54	.0794	6.6	.052	SI
1304.	35.	5.	5.	2488.5422	-6.47	184.65	77.47	16.54	.0787	6.6	.052	SI
>1304.	0.	5.	5.	2488.5422	-6.47	184.68	77.47	16.6	.0787	6.72	.053	SI
1744.	440.	5.	5.	2086.9109	-5.42	154.41	77.47	15.26	.0648	5.48	.036	SI
>1744.	0.	5.	5.	2086.9109	-5.42	154.41	77.47	15.27	.0648	5.49	.036	SI
1779.	35.	5.	5.	2041.2186	-5.3	151.02	77.47	15.24	.0632	5.44	.034	SI
>1779.	0.	5.	5.	2041.2141	-5.3	151.02	77.47	15.24	.0632	5.44	.034	SI
1814.	35.	5.	5.	1962.6681	-5.09	145.2	77.47	15.24	.0604	5.44	.033	SI
>1814.	0.	5.	5.	1962.6681	-5.09	145.23	77.47	15.3	.0604	5.56	.034	SI
2001.	187.	5.	5.	1503.7961	-4.03	119.68	66.85	11.93	.0488	14.63	.071	SI
2254.	440.	5.	3.	805.3116	-2.1	76.44	56.23	9.18	.0286	14.24	.041	SI
>2254.	0.	5.	3.	805.3116	-2.1	76.44	56.23	9.18	.0286	14.25	.041	SI
2289.	35.	5.	3.	699.3321	-1.75	65.96	56.23	9.15	.0236	14.23	.034	SI
>2289.	0.	5.	3.	699.3277	-1.75	65.96	56.23	9.15	.0236	14.23	.034	SI
2324.	35.	5.	3.	567.5782	-1.42	53.53	56.23	9.15	.0177	14.23	.025	SI
>2324.	0.	5.	3.	567.5782	-1.42	53.53	56.23	9.16	.0177	14.24	.025	SI

2434.	110.	5.	2.	136.283	!	- .36	14.96	45.62	5.78	.0043	13.33	.006	SI
>2434.	0.	5.	2.	136.283	!	- .36	14.96	45.62	5.79	.0043	13.33	.006	SI
2469.	35.	5.	2.	-3.9567	!	- .01	.43	56.99	36.29	.0001	32.57	0.	SI
>2469.	0.	5.	2.	-3.9567	!	- .01	.34	56.99	36.29	.0001	32.57	0.	SI
2504.	35.	5.	1.	-.9553	!	0.	.1	56.99	25.75	0.	26.08	0.	SI
>2504.	0.	5.	1.	-.9553	!	0.	.1	56.99	25.75	0.	26.07	0.	SI
2529.	25.	5.	1.	-.1328	!	0.	.01	56.99	25.88	0.	26.15	0.	SI

TENSIONI DI ESERCIZIO E FESSURAZIONE - QUASI PERMANENTI:

Progressive	Se	Ar	Momento	σc	σf	As	hc,ef	Eps%	Sr,max	wd	Ve		
> 9.	9.	5.	1.	-.1328	!	0.	.01	41.28	7.54	0.	12.63	0.	SI
9.	9.	5.	1.	-.1328	!	0.	.01	41.28	7.54	0.	12.63	0.	SI
34.	34.	5.	1.	-.9553	!	0.	.1	41.28	7.67	0.	12.92	0.	SI
> 34.	0.	5.	1.	-.9553	!	0.	.1	41.28	7.67	0.	12.91	0.	SI
54.	20.	5.	2.	-2.4467	!	- .01	.25	41.28	8.6	.0001	13.78	0.	SI
60.	26.	5.	2.	-2.8985	!	- .01	.3	56.99	23.86	.0001	20.74	0.	SI
69.	35.	5.	2.	-3.1959	!	- .01	.34	56.99	30.1	.0001	24.6	0.	SI
> 69.	0.	5.	2.	-3.1959	!	- .01	.42	56.99	30.1	.0001	24.6	0.	SI
78.	9.	5.	2.	32.3219	!	- .08	3.65	45.62	11.96	.001	11.98	.001	SI
84.	15.	5.	2.	53.6215	!	- .14	6.04	45.62	11.96	.0017	11.98	.002	SI
104.	35.	5.	2.	129.872	!	- .35	14.44	45.62	11.96	.0041	11.98	.005	SI
> 104.	0.	5.	2.	129.872	!	- .35	14.44	45.62	11.96	.0041	11.99	.005	SI
214.	110.	5.	3.	540.8088	!	-1.36	51.53	56.23	14.38	.0147	11.2	.016	SI
> 214.	0.	5.	3.	540.8088	!	-1.36	51.53	56.23	14.38	.0147	11.21	.017	SI
249.	35.	5.	3.	666.1589	!	-1.68	63.47	56.23	14.35	.0199	11.15	.022	SI
> 249.	0.	5.	3.	666.1589	!	-1.68	63.47	56.23	14.35	.0199	11.15	.022	SI
284.	35.	5.	3.	767.8276	!	-2.02	73.7	56.23	14.35	.0248	11.15	.028	SI
> 284.	0.	5.	3.	767.8276	!	-2.02	73.71	56.23	14.42	.0247	11.28	.028	SI
724.	440.	5.	5.	1870.8861	!	-4.88	139.24	77.47	17.86	.0566	8.83	.05	SI
> 724.	0.	5.	5.	1870.8861	!	-4.88	139.24	77.47	17.87	.0566	8.84	.05	SI
759.	35.	5.	5.	1945.1094	!	-5.07	144.76	77.47	17.84	.0593	8.79	.052	SI
> 759.	0.	5.	5.	1945.1094	!	-5.07	144.76	77.47	17.84	.0593	8.79	.052	SI
794.	35.	5.	5.	1989.2531	!	-5.19	148.04	77.47	17.84	.0608	8.79	.053	SI
> 794.	0.	5.	5.	1989.2531	!	-5.19	148.07	77.47	17.9	.0608	8.91	.054	SI
851.	57.	5.	5.	2048.7562	!	-5.34	152.44	77.47	17.73	.063	8.62	.054	SI
1234.	440.	5.	5.	2371.2843	!	-6.17	175.96	77.47	16.56	.0746	6.65	.05	SI
>1234.	0.	5.	5.	2371.2843	!	-6.17	175.97	77.47	16.57	.0746	6.66	.05	SI
1260.	26.	5.	5.	2383.5722	!	-6.2	176.87	77.47	16.54	.075	6.62	.05	SI
1269.	35.	5.	5.	2387.9297	!	-6.21	177.19	77.47	16.54	.0752	6.6	.05	SI
>1269.	0.	5.	5.	2387.9297	!	-6.21	177.19	77.47	16.54	.0752	6.6	.05	SI
1304.	35.	5.	5.	2371.2943	!	-6.17	175.95	77.47	16.54	.0746	6.6	.049	SI
>1304.	0.	5.	5.	2371.2943	!	-6.17	175.98	77.47	16.6	.0746	6.72	.05	SI
1744.	440.	5.	5.	1989.246	!	-5.16	147.18	77.47	15.26	.0614	5.48	.034	SI
>1744.	0.	5.	5.	1989.246	!	-5.16	147.18	77.47	15.27	.0614	5.49	.034	SI
1779.	35.	5.	5.	1945.1123	!	-5.05	143.91	77.47	15.24	.0598	5.44	.033	SI
>1779.	0.	5.	5.	1945.1123	!	-5.05	143.91	77.47	15.24	.0598	5.44	.033	SI
1814.	35.	5.	5.	1870.894	!	-4.85	138.42	77.47	15.24	.0572	5.44	.031	SI
>1814.	0.	5.	5.	1870.894	!	-4.86	138.44	77.47	15.3	.0572	5.56	.032	SI
2001.	187.	5.	5.	1435.1328	!	-3.84	114.22	66.85	11.93	.0462	14.63	.068	SI
2254.	440.	5.	3.	767.8165	!	-2.01	72.88	56.23	9.18	.0269	14.24	.038	SI
>2254.	0.	5.	3.	767.8165	!	-2.01	72.88	56.23	9.18	.0269	14.25	.038	SI
2289.	35.	5.	3.	666.1643	!	-1.67	62.83	56.23	9.15	.0221	14.23	.032	SI
>2289.	0.	5.	3.	666.1643	!	-1.67	62.83	56.23	9.15	.0221	14.23	.032	SI
2324.	35.	5.	3.	540.8241	!	-1.35	51.01	56.23	9.15	.0165	14.23	.024	SI
>2324.	0.	5.	3.	540.8241	!	-1.35	51.01	56.23	9.16	.0165	14.24	.024	SI
2434.	110.	5.	2.	129.8621	!	- .34	14.25	45.62	5.78	.0041	13.33	.005	SI
>2434.	0.	5.	2.	129.8621	!	- .34	14.25	45.62	5.79	.0041	13.33	.005	SI
2469.	35.	5.	2.	-3.945	!	- .01	.42	56.99	36.29	.0001	32.57	0.	SI
>2469.	0.	5.	2.	-3.1959	!	- .01	.34	56.99	36.29	.0001	32.57	0.	SI
2504.	35.	5.	1.	-.9553	!	0.	.1	56.99	25.75	0.	26.08	0.	SI
>2504.	0.	5.	1.	-.9553	!	0.	.1	56.99	25.75	0.	26.07	0.	SI
2529.	25.	5.	1.	-.1328	!	0.	.01	56.99	25.88	0.	26.15	0.	SI

## VERIFICA TRAVI SUD

### PARAMETRI GENERALI

Metodo di verifica : stati limite - NTC18 ->  
 Duttilita' : non prevista (struttura non dissipativa)  
 Unita' di misura : cm; kN; kN/m; kNm; N/mm2; deform. %.  
 Unita' particolari : fessure [wk]:mm - ferri:mm e cm2 - sezioni:cm e derivate.  
 Copriferri (assi) : longitudinali= 3 ; staffe= 2

### CARATTERISTICHE MATERIALI

CLS : Rck=30. ; fck=24.9 ; fctk= 1.79; fctm= 2.56; Ec= 31447.2 ;  
 gc =1.5 ; fcd=14.11; fbd= 2.69; fctd= 1.19; Ecu=.2% (limit.elastico)  
 ACCIAIO : B450C; ftk=517.5 ; fyk=450. ; Es=210000. ;  
 gs =1.15; fyd=391.3 ; ftd(k\*fyd)=450. ; fud=443.98; Eud=.19% (limit.elastico)  
 Fibre : Tipo = FRMC in PBO ; Samm= 1036.3 ; n=12.45 ; (valutate alle TA)

### VALORI TENSIONI MASSIME IN ESERCIZIO

GRUPPO : aggressivo.  
 CLS : σc (rara)=14.94; σc (quasi permanente)=11.2 ; fbd(esercizio)= 2.69  
 ACCIAIO : σf (rara)=360. ; Coeff.Omogeneizzazione= 15  
 FESSURE : wmax(fre.)=.3 ; wmax(q.p.)=.2 [4.1.2.2.4.5]; kt=.4 [EN 1992-1 7.3.4].

### CASI DI CARICO

SLU			FREQUENTI			QUASI PERMANENTI		
Nome	Descrizione	Sest	Nome	Descrizione	Sest	Nome	Descrizione	Sest
1.	SLU	1.	18.	Frequente	1.	20.	Quasi Perm	1.
2.	SLU VENTYOY	2.	19.	Frequente Ventoy	2.			
6.	SLU con SISMAX	PRINC32						
7.	SLU con SISMAZ	PRINC32						
8.	SLU con SISMAZ	PRINC32						
13.	SLU Solo Perm.	1.						

### SEZIONI UTILIZZATE

5) Sezione a Z (2): 90/20/57X190/25/25; A=6475.; Jg=29062264.; E=31447.16

### ARMATURE LONGITUDINALI (%=100\*Af/Acls - Acls=area intera sezione)

Nro	Totale	%	Super.	%	Barre	Infer.	%	Barre
1	88.91	1.373	46.43	.717	2d20 +2d26 +2d2 ...	42.47	.656	4d26 +4d26
2	91.92	1.42	47.44	.733	2d20 +2d26 +2d2 ...	44.48	.687	4d26 +4d26 +2d8 ...
3	95.94	1.482	47.44	.733	2d20 +2d26 +2d2 ...	48.51	.749	4d26 +1d16 +4d2 ...
4	95.94	1.482	36.82	.569	2d20 +2d26 +2d2 ...	59.12	.913	4d26 +2d26 +1d1 ...
5	95.94	1.482	26.2	.405	2d20 +2d26 +1d8 ...	69.74	1.077	4d26 +2d26 +2d2 ...
6	95.94	1.482	15.58	.241	2d20 +1d8 +1d20 ...	80.36	1.241	4d26 +2d26 +2d2 ...
F	5.00	-	-	-	Long.Faccia esterna	-	-	Long.Faccia esterna

## TRAVE TIPO:

DESCRIZIONE CAMPATE											
Cam.	Descriz.	S.ini	Sez.	S.fin	Incl.	L.assi	L.net.	lambda	K	r.Ar.	lam.max
1	A4155	5	5	5	0	34.	34.	.179	.4	5.	29.848
2	A4156	5	5	5	0	35.	20.	.184	1.5	1.	22.386
3	A4157	5	5	5	0	35.	20.	.184	1.5	5.	107.842
4	A4158	5	5	5	0	110.	110.	.579	1.5	4.322	93.215
5	A4159	5	5	5	0	35.	35.	.184	1.5	3.862	83.298
6	A4160	5	5	5	0	35.	35.	.184	1.5	3.358	72.42
7	A4161	5	5	5	0	440.	440.	2.316	1.5	1.737	37.473
8	A4162	5	5	5	0	35.	35.	.184	1.5	1.67	36.011
9	A4163	5	5	5	0	35.	35.	.184	1.5	1.633	35.225
10	A4164	5	5	5	0	440.	440.	2.316	1.5	1.369	29.529
11	A4165	5	5	5	0	35.	35.	.184	1.5	1.359	29.319
12	A4166	5	5	5	0	35.	35.	.184	1.5	1.359	29.321
13	A4167	5	5	5	0	440.	440.	2.316	1.5	1.37	29.542
14	A4168	5	5	5	0	35.	35.	.184	1.5	1.642	35.42
15	A4169	5	5	5	0	35.	35.	.184	1.5	1.679	36.215
16	A4170	5	5	5	0	440.	440.	2.316	1.5	1.748	37.693
17	A4171	5	5	5	0	35.	35.	.184	1.5	3.36	72.461
18	A4172	5	5	5	0	35.	35.	.184	1.5	3.899	84.099
19	A4173	5	5	5	0	110.	110.	.579	1.5	4.807	103.687
20	A4174	5	5	5	0	35.	20.	.184	1.5	5.	107.842
21	A4175	5	5	5	0	35.	20.	.184	1.5	1.	22.386
22	A4176	5	5	5	0	34.	34.	.179	.4	1.	5.97

### VERIFICHE ALLO STATO LIMITE ULTIMO

#### FLESSIONE:

Progressive	SE	Ar	Msd	Epsc	Epsac	Mrd	Epsc	Epsac	Cam	x/d	Mr/Ms	VE
> 17.	17.	5.1.	-1.2852	0.	0.	-3659.709	-.09	.186	2.	.325	12848.	!SI
34.	34.	5.1.	-1.2852	0.	0.	-3659.265	-.09	.186	2.	.325	12847.	!SI
> 34.	0.	5.1.	-4.2884	0.	0.	-3659.292	-.09	.186	2.	.325	1853.3	!SI
54.	20.	5.2.	-4.2884	0.	0.	-3847.35	-.093	.186	2.	.333	1897.1	!SI
69.	35.	5.2.	-4.2884	0.	0.	-3661.463	-.091	.186	2.	.327	1853.8	!SI
> 69.	0.	5.2.	-5.4382	0.	0.	-3661.463	-.091	.186	2.	.327	1673.3	!SI
69.	0.	5.2.	206.1093	-.004	.011	3457.5996	-.067	.186	2.	.265	16.78	!SI
104.	35.	5.2.	-5.4382	0.	0.	-3346.856	-.086	.186	2.	.315	1615.4	!SI
104.	35.	5.2.	206.1093	-.004	.011	3502.3743	-.071	.186	2.	.276	16.99	!SI
> 104.	0.	5.2.	706.4734	-.013	.037	3502.3324	-.071	.186	2.	.276	14.957	!SI
141.	37.	5.3.	858.069	-.016	.043	3708.429	-.075	.186	2.	.287	14.322	!SI
214.	110.	5.3.	858.069	-.015	.039	4085.9231	-.079	.186	2.	.299	14.762	!SI
> 214.	0.	5.3.	1057.9641	-.019	.048	4085.8904	-.079	.186	2.	.299	13.862	!SI
249.	35.	5.3.	1057.9641	-.019	.048	4086.1166	-.079	.186	2.	.299	13.862	!SI
> 249.	0.	5.3.	1216.9318	-.022	.055	4086.1169	-.079	.186	2.	.299	13.358	!SI
284.	35.	5.3.	1216.9318	-.023	.056	4045.741	-.083	.186	2.	.308	13.325	!SI
> 284.	0.	5.3.	1574.3482	-.03	.072	4045.1748	-.083	.186	2.	.308	12.569	!SI
668.	384.	5.5.	2978.6418	-.06	.106	5175.0923	-.113	.186	2.	.378	11.737	!SI
724.	440.	5.5.	2978.6418	-.06	.106	5176.9997	-.113	.186	2.	.378	11.738	!SI
> 724.	0.	5.5.	3100.6573	-.062	.111	5176.9436	-.113	.186	2.	.378	11.67	!SI
759.	35.	5.5.	3100.6573	-.062	.111	5177.2829	-.113	.186	2.	.378	11.67	!SI



> 759.	0.	5.	5.	3170.0868!	-.064!	.113!	5177.2833!	-.113!	.186!	2.	.378!	1.633!	SI
794.	35.	5.	5.	3170.0868!	-.064!	.113!	5177.2833!	-.113!	.186!	2.	.378!	1.633!	SI
> 794.	0.	5.	5.	3312.9694!	-.067!	.118!	5176.512!	-.113!	.186!	2.	.378!	1.562!	SI
1150.	356.	5.	5.	3789.8256!	-.078!	.135!	5188.6004!	-.114!	.186!	2.	.379!	1.369!	SI
1234.	440.	5.	5.	3789.8256!	-.078!	.135!	5191.4635!	-.114!	.186!	2.	.379!	1.37!	SI
>1234.	0.	5.	5.	3819.078!	-.079!	.136!	5191.4072!	-.114!	.186!	2.	.379!	1.359!	SI
1269.	35.	5.	5.	3819.078!	-.079!	.136!	5191.7476!	-.114!	.186!	2.	.379!	1.359!	SI
>1269.	0.	5.	5.	3819.0782!	-.079!	.136!	5191.7479!	-.114!	.186!	2.	.379!	1.359!	SI
1304.	35.	5.	5.	3819.0782!	-.079!	.136!	5191.7479!	-.114!	.186!	2.	.379!	1.359!	SI
>1304.	0.	5.	5.	3789.8434!	-.078!	.135!	5190.9742!	-.114!	.186!	2.	.379!	1.37!	SI
1744.	440.	5.	5.	3312.9626!	-.067!	.118!	5205.9716!	-.114!	.186!	2.	.379!	1.571!	SI
>1744.	0.	5.	5.	3170.074!	-.064!	.113!	5205.9152!	-.114!	.186!	2.	.379!	1.642!	SI
1779.	35.	5.	5.	3170.074!	-.064!	.113!	5206.2566!	-.114!	.186!	2.	.379!	1.642!	SI
>1779.	0.	5.	5.	3100.6622!	-.062!	.11!	5206.2569!	-.114!	.186!	2.	.379!	1.679!	SI
1814.	35.	5.	5.	3100.6622!	-.062!	.11!	5206.2569!	-.114!	.186!	2.	.379!	1.679!	SI
>1814.	0.	5.	5.	2978.6562!	-.059!	.106!	5205.4809!	-.114!	.186!	2.	.379!	1.748!	SI
1934.	120.	5.	5.	2844.9501!	-.056!	.101!	5209.5688!	-.114!	.186!	2.	.379!	1.831!	SI
2254.	440.	5.	3.	1574.3363!	-.03!	.071!	4088.3734!	-.084!	.186!	2.	.31!	2.597!	SI
>2254.	0.	5.	3.	1216.9134!	-.023!	.055!	4088.3315!	-.084!	.186!	2.	.31!	3.36!	SI
2289.	35.	5.	3.	1216.9134!	-.022!	.055!	4125.229!	-.08!	.186!	2.	.3!	3.39!	SI
>2289.	0.	5.	3.	1057.9735!	-.019!	.048!	4125.2292!	-.08!	.186!	2.	.3!	3.899!	SI
2324.	35.	5.	3.	1057.9735!	-.019!	.048!	4125.2292!	-.08!	.186!	2.	.3!	3.899!	SI
>2324.	0.	5.	3.	858.0958!	-.015!	.039!	4125.1824!	-.08!	.186!	2.	.3!	4.807!	SI
2397.	73.	5.	3.	858.0958!	-.016!	.042!	3740.7773!	-.076!	.186!	2.	.289!	4.359!	SI
2434.	110.	5.	2.	706.4719!	-.013!	.037!	3542.65!	-.072!	.186!	2.	.278!	5.015!	SI
>2434.	0.	5.	2.	-5.4382!	0.	0.	-3319.421!	-.084!	.186!	2.	.312!	610.4!	SI
2434.	0.	5.	2.	206.0919!	-.004!	.011!	3542.6184!	-.072!	.186!	2.	.278!	17.19!	SI
2469.	35.	5.	2.	-5.4382!	0.	0.	-3630.478!	-.09!	.186!	2.	.325!	667.6!	SI
2469.	35.	5.	2.	206.0919!	-.004!	.011!	3497.1399!	-.068!	.186!	2.	.268!	16.97!	SI
>2469.	0.	5.	2.	-4.2885!	0.	0.	-3630.478!	-.09!	.186!	2.	.325!	846.6!	SI
2484.	15.	5.	2.	-4.2885!	0.	0.	-3813.725!	-.092!	.186!	2.	.33!	889.3!	SI
2504.	35.	5.	1.	-4.2885!	0.	0.	-3629.632!	-.089!	.186!	2.	.323!	846.4!	SI
>2504.	0.	5.	1.	-1.2852!	0.	0.	-3629.645!	-.089!	.186!	2.	.323!	2824.!	SI
2521.	17.	5.	1.	-1.2852!	0.	0.	-3629.229!	-.089!	.186!	2.	.323!	2824.!	SI

**TAGLIO:**

Progressive	Se	Vsd	VRd	VRcd	VRsd	Asw	s	ctgT	Ve						
> 0.	0.	5.	0.	99.85	921.79	926.89	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO
34.	34.	5.	-7.56!	99.85!	921.79!	926.89!	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO
> 34.	0.	5.	-7.56!	99.85!	921.79!	926.89!	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO
69.	35.	5.	-12.01!	99.85!	921.79!	926.89!	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO
> 69.	0.	5.	607.57!	144.02!	921.79!	926.89!	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO
104.	35.	5.	599.79!	181.45!	921.79!	926.89!	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO
> 104.	0.	5.	599.7!	181.45!	921.79!	926.89!	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO
141.	37.	5.	591.48!	195.46!	921.79!	926.89!	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO
214.	110.	5.	575.02!	195.46!	921.79!	926.89!	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO
> 214.	0.	5.	575.1!	195.46!	921.79!	926.89!	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO
249.	35.	5.	567.32!	195.46!	921.79!	926.89!	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO
> 249.	0.	5.	457.96!	195.46!	921.79!	926.89!	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO
284.	35.	5.	450.18!	216.29!	921.79!	926.89!	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO
> 284.	0.	5.	450.16!	216.29!	921.79!	926.89!	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO
340.	56.	5.	437.68!	219.12!	921.79!	926.89!	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO
500.	226.	5.	437.69!	219.12!	921.79!	926.89!	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO
724.	214.	5.	352.56!	219.12!	818.87!	501.56!	1.01	33.	2.5	SI					
> 724.	0.	5.	352.56!	219.12!	818.87!	501.56!	1.01	33.	2.5	SI					
759.	35.	5.	344.78!	219.12!	818.87!	501.56!	1.01	33.	2.5	SI					
> 759.	0.	5.	202.1!	219.12!	818.87!	501.56!	1.01	33.	2.5	SI					
794.	35.	5.	194.32!	219.12!	818.87!	501.56!	1.01	33.	2.5	SI					
> 794.	0.	5.	194.31!	219.12!	818.87!	501.56!	1.01	33.	2.5	SI					
1234.	440.	5.	87.48!	219.12!	818.87!	501.56!	1.01	33.	2.5	SI					
>1234.	0.	5.	87.48!	219.12!	818.87!	501.56!	1.01	33.	2.5	SI					
1269.	35.	5.	79.7!	219.12!	818.87!	501.56!	1.01	33.	2.5	SI					
>1269.	0.	5.	-79.65!	219.12!	818.87!	501.56!	1.01	33.	2.5	SI					
1304.	35.	5.	-87.43!	219.12!	818.87!	501.56!	1.01	33.	2.5	SI					
>1304.	0.	5.	-87.48!	219.12!	818.87!	501.56!	1.01	33.	2.5	SI					
1744.	440.	5.	-194.32!	219.12!	818.87!	501.56!	1.01	33.	2.5	SI					
>1744.	0.	5.	-194.27!	219.12!	818.87!	501.56!	1.01	33.	2.5	SI					
1779.	35.	5.	-202.05!	219.12!	818.87!	501.56!	1.01	33.	2.5	SI					
>1779.	0.	5.	-344.73!	219.12!	818.87!	501.56!	1.01	33.	2.5	SI					
1814.	35.	5.	-352.51!	219.12!	818.87!	501.56!	1.01	33.	2.5	SI					
>1814.	0.	5.	-352.55!	219.12!	818.87!	501.56!	1.01	33.	2.5	SI					
1814.	214.	5.	-352.55!	219.12!	818.87!	501.56!	1.01	33.	2.5	SI					
2028.	226.	5.	-450.13!	216.29!	921.79!	926.89!	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO
>2254.	0.	5.	-450.13!	216.29!	921.79!	926.89!	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO
2289.	35.	5.	-457.91!	195.46!	921.79!	926.89!	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO
>2289.	0.	5.	-567.27!	195.46!	921.79!	926.89!	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO
2324.	35.	5.	-575.05!	195.46!	921.79!	926.89!	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO
>2324.	0.	5.	-575.06!	195.46!	921.79!	926.89!	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO
2434.	110.	5.	-599.75!	181.45!	921.79!	926.89!	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO
>2434.	0.	5.	-599.74!	181.45!	921.79!	926.89!	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO
2469.	35.	5.	-607.52!	99.85!	921.79!	926.89!	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO
>2469.	0.	5.	12.01!	99.85!	921.79!	926.89!	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO
2504.	35.	5.	7.56!	99.85!	921.79!	926.89!	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO
>2504.	0.	5.	7.56!	99.85!	921.79!	926.89!	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO
2538.	34.	5.	0.	99.85!	921.79!	926.89!	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO
2538.	34.	5.	0.	99.85!	921.79!	926.89!	1.01	15.	2.1	SI	Fasciatura	con	FRCM	in	PBO

**VERIFICHE ALLO STATO LIMITE DI ESERCIZIO**

**TENSIONI DI ESERCIZIO E FESSURAZIONE - RARE:**

Progressive	Se	Ar	Momento	σc	σf	As	hc,ef	Eps%	Sr,max	wd	Ve	
> 17.	17.	5.	1.	-246!	0.	.03!	41.28!	7.58!	0.	12.72!	0.	SI
34.	34.	5.	1.	-9842!	0.	.1!	41.28!	7.67!	0.	12.92!	0.	SI
> 34.	0.	5.	1.	-9842!	0.	.1!	41.28!	7.67!	0.	12.91!	0.	SI
69.	35.	5.	2.	-3.284!	-.01!	.35!	56.99!	30.1!	.0001	24.59!	0.	SI
> 69.	0.	5.	2.	-4.15!	-.01!	.44!	56.99!	30.1!	.0001	24.59!	0.	SI
104.	35.	5.	2.	146.7793!	-.39!	16.32!	45.62!	11.96!	.0047	11.98!	.006!	SI
> 104.	0.	5.	2.	146.7793!	-.39!	16.32!	45.62!	11.96!	.0047	11.98!	.006!	SI
141.	37.	5.	3.	303.2878!	-.8!	31.83!	45.62!	11.8!	.0091	11.68!	.011!	SI

177.	73.	5.	3.	458.0347	-1.15	43.67	56.23	14.58	.0125	11.51	.014	SI
214.	110.	5.	3.	611.0209	-1.54	58.22	56.23	14.38	.0174	11.2	.019	SI
> 214.	0.	5.	3.	611.0209	-1.54	58.22	56.23	14.38	.0174	11.21	.019	SI
232.	18.	5.	3.	682.2992	-1.72	65.01	56.23	14.37	.0206	11.18	.023	SI
249.	35.	5.	3.	753.056	-1.89	71.75	56.23	14.35	.0238	11.15	.027	SI
> 249.	0.	5.	3.	753.0927	-1.89	71.76	56.23	14.35	.0238	11.15	.027	SI
284.	35.	5.	3.	867.2254	-2.28	83.24	56.23	14.35	.0293	11.15	.033	SI
> 284.	0.	5.	3.	867.2254	-2.28	83.25	56.23	14.42	.0293	11.28	.033	SI
724.	440.	5.	5.	2121.5032	-5.53	157.89	77.47	17.86	.0655	8.83	.058	SI
> 724.	0.	5.	5.	2121.5032	-5.53	157.9	77.47	17.87	.0655	8.84	.058	SI
759.	35.	5.	5.	2207.3816	-5.76	164.28	77.47	17.84	.0686	8.79	.06	SI
> 759.	0.	5.	5.	2207.4192	-5.76	164.28	77.47	17.84	.0686	8.79	.06	SI
794.	35.	5.	5.	2257.6223	-5.89	168.01	77.47	17.84	.0704	8.79	.062	SI
> 794.	0.	5.	5.	2257.6223	-5.89	168.04	77.47	17.9	.0703	8.91	.063	SI
850.	56.	5.	5.	2325.8869	-6.07	173.06	77.47	17.73	.0728	8.62	.063	SI
1234.	440.	5.	5.	2697.4591	-7.02	200.17	77.47	16.56	.0861	6.65	.057	SI
>1234.	0.	5.	5.	2697.4591	-7.02	200.17	77.47	16.57	.0861	6.66	.057	SI
1269.	35.	5.	5.	2717.2515	-7.07	201.63	77.47	16.54	.0868	6.6	.057	SI
>1269.	0.	5.	5.	2717.2516	-7.07	201.63	77.47	16.54	.0868	6.6	.057	SI
1304.	35.	5.	5.	2697.4718	-7.02	200.16	77.47	16.54	.0861	6.6	.057	SI
>1304.	0.	5.	5.	2697.4718	-7.02	200.19	77.47	16.6	.0861	6.72	.058	SI
1744.	440.	5.	5.	2257.6132	-5.86	167.04	77.47	15.26	.0708	5.48	.039	SI
>1744.	0.	5.	5.	2257.6132	-5.86	167.04	77.47	15.27	.0708	5.49	.039	SI
1779.	35.	5.	5.	2207.4227	-5.73	163.31	77.47	15.24	.0691	5.44	.038	SI
>1779.	0.	5.	5.	2207.3851	-5.73	163.31	77.47	15.24	.0691	5.44	.038	SI
1814.	35.	5.	5.	2121.5135	-5.51	156.96	77.47	15.24	.066	5.44	.036	SI
>1814.	0.	5.	5.	2121.5135	-5.51	156.98	77.47	15.3	.066	5.56	.037	SI
2004.	190.	5.	5.	1614.1557	-4.33	129.05	66.85	11.92	.0532	14.63	.078	SI
2254.	440.	5.	3.	867.2122	-2.26	82.32	56.23	9.18	.0314	14.24	.045	SI
>2254.	0.	5.	3.	867.2122	-2.26	82.32	56.23	9.18	.0314	14.25	.045	SI
2289.	35.	5.	3.	753.0994	-1.89	71.03	56.23	9.15	.026	14.23	.037	SI
>2289.	0.	5.	3.	753.0627	-1.89	71.03	56.23	9.15	.026	14.23	.037	SI
2324.	35.	5.	3.	611.0401	-1.53	57.63	56.23	9.15	.0197	14.23	.028	SI
>2324.	0.	5.	3.	611.0401	-1.53	57.63	56.23	9.16	.0197	14.24	.028	SI
2434.	110.	5.	2.	146.7669	-.39	16.12	45.62	5.78	.0046	13.33	.006	SI
>2434.	0.	5.	2.	146.7669	-.39	16.12	45.62	5.79	.0046	13.33	.006	SI
2469.	35.	5.	2.	-4.15	-.01	.45	56.99	36.13	.0001	32.47	0.	SI
>2469.	0.	5.	2.	-3.284	-.01	.35	56.99	36.13	.0001	32.47	0.	SI
2504.	35.	5.	1.	-.9842	0.	.11	56.99	25.75	0.	26.07	0.	SI
>2504.	0.	5.	1.	-.9842	0.	.11	56.99	25.75	0.	26.07	0.	SI
2521.	17.	5.	1.	-.246	0.	.03	56.99	25.84	0.	26.13	0.	SI

TENSIONI DI ESERCIZIO E FESSURAZIONE - FREQUENTI:

Progressive	Se	Ar	Momento	$\sigma_c$	$\sigma_f$	As	hc,ef	Eps%	Sr,max	wd	Ve	
> 17.	17.	5.	1.	-.2388	0.	.03	41.28	7.58	0.	12.72	0.	SI
34.	34.	5.	1.	-.9553	0.	.1	41.28	7.67	0.	12.92	0.	SI
> 34.	0.	5.	1.	-.9553	0.	.1	41.28	7.67	0.	12.91	0.	SI
69.	35.	5.	2.	-3.1875	-.01	.34	56.99	30.1	.0001	24.59	0.	SI
> 69.	0.	5.	2.	-3.9537	-.01	.42	56.99	30.1	.0001	24.59	0.	SI
104.	35.	5.	2.	136.2478	-.36	15.15	45.62	11.96	.0043	11.99	.005	SI
> 104.	0.	5.	2.	136.2478	-.36	15.15	45.62	11.96	.0043	11.99	.005	SI
141.	37.	5.	3.	281.656	-.74	29.56	45.62	11.8	.0084	11.68	.01	SI
177.	73.	5.	3.	425.3549	-1.07	40.55	56.23	14.58	.0116	11.51	.013	SI
214.	110.	5.	3.	567.3454	-1.43	54.06	56.23	14.38	.0154	11.2	.017	SI
> 214.	0.	5.	3.	567.3454	-1.43	54.06	56.23	14.38	.0154	11.21	.017	SI
232.	18.	5.	3.	633.4571	-1.59	60.36	56.23	14.37	.0184	11.18	.021	SI
249.	35.	5.	3.	699.0627	-1.76	66.61	56.23	14.35	.0214	11.15	.024	SI
> 249.	0.	5.	3.	699.07	-1.76	66.61	56.23	14.35	.0214	11.15	.024	SI
284.	35.	5.	3.	805.0242	-2.11	77.27	56.23	14.35	.0265	11.15	.029	SI
> 284.	0.	5.	3.	805.0242	-2.12	77.28	56.23	14.42	.0264	11.28	.03	SI
724.	440.	5.	5.	1961.9542	-5.12	146.02	77.47	17.86	.0599	8.83	.053	SI
> 724.	0.	5.	5.	1961.9542	-5.12	146.02	77.47	17.87	.0599	8.84	.053	SI
759.	35.	5.	5.	2040.4841	-5.32	151.86	77.47	17.84	.0627	8.79	.055	SI
> 759.	0.	5.	5.	2040.4916	-5.32	151.86	77.47	17.84	.0627	8.79	.055	SI
794.	35.	5.	5.	2086.173	-5.44	155.26	77.47	17.84	.0643	8.79	.057	SI
> 794.	0.	5.	5.	2086.173	-5.44	155.28	77.47	17.9	.0643	8.91	.057	SI
850.	56.	5.	5.	2147.5592	-5.6	159.79	77.47	17.73	.0665	8.62	.057	SI
1234.	440.	5.	5.	2487.6486	-6.47	184.6	77.47	16.56	.0787	6.65	.052	SI
>1234.	0.	5.	5.	2487.6486	-6.47	184.6	77.47	16.57	.0787	6.66	.052	SI
1269.	35.	5.	5.	2505.8526	-6.52	185.94	77.47	16.54	.0794	6.6	.052	SI
>1269.	0.	5.	5.	2505.8526	-6.52	185.94	77.47	16.54	.0794	6.6	.052	SI
1304.	35.	5.	5.	2487.6599	-6.47	184.59	77.47	16.54	.0787	6.6	.052	SI
>1304.	0.	5.	5.	2487.6599	-6.47	184.62	77.47	16.6	.0787	6.72	.053	SI
1744.	440.	5.	5.	2086.165	-5.41	154.35	77.47	15.26	.0648	5.48	.036	SI
>1744.	0.	5.	5.	2086.165	-5.41	154.35	77.47	15.27	.0648	5.49	.036	SI
1779.	35.	5.	5.	2040.4948	-5.29	150.96	77.47	15.24	.0632	5.44	.034	SI
>1779.	0.	5.	5.	2040.4873	-5.29	150.96	77.47	15.24	.0632	5.44	.034	SI
1814.	35.	5.	5.	1961.9635	-5.09	145.15	77.47	15.24	.0604	5.44	.033	SI
>1814.	0.	5.	5.	1961.9635	-5.09	145.18	77.47	15.3	.0604	5.56	.034	SI
2004.	190.	5.	5.	1495.6964	-4.01	119.58	66.85	11.92	.0487	14.63	.071	SI
2254.	440.	5.	3.	805.0122	-2.1	76.41	56.23	9.18	.0286	14.24	.041	SI
>2254.	0.	5.	3.	805.0122	-2.1	76.41	56.23	9.18	.0286	14.25	.041	SI
2289.	35.	5.	3.	699.0761	-1.75	65.93	56.23	9.15	.0236	14.23	.034	SI
>2289.	0.	5.	3.	699.0688	-1.75	65.93	56.23	9.15	.0236	14.23	.034	SI
2324.	35.	5.	3.	567.3626	-1.42	53.51	56.23	9.15	.0177	14.23	.025	SI
>2324.	0.	5.	3.	567.3626	-1.42	53.51	56.23	9.16	.0177	14.24	.025	SI
2434.	110.	5.	2.	136.2366	-.36	14.96	45.62	5.78	.0043	13.33	.006	SI
>2434.	0.	5.	2.	136.2366	-.36	14.96	45.62	5.79	.0043	13.33	.006	SI
2469.	35.	5.	2.	-3.9537	-.01	.42	56.99	36.13	.0001	32.47	0.	SI
>2469.	0.	5.	2.	-3.1876	-.01	.34	56.99	36.13	.0001	32.47	0.	SI
2504.	35.	5.	1.	-.9553	0.	.1	56.99	25.75	0.	26.07	0.	SI
>2504.	0.	5.	1.	-.9553	0.	.1	56.99	25.75	0.	26.07	0.	SI
2521.	17.	5.	1.	-.2388	0.	.03	56.99	25.84	0.	26.13	0.	SI

TENSIONI DI ESERCIZIO E FESSURAZIONE - QUASI PERMANENTI:

Progressive	Se	Ar	Momento	$\sigma_c$	$\sigma_f$	As	hc,ef	Eps%	Sr,max	wd	Ve	
> 17.	17.	5.	1.	-.2388	0.	.03	41.28	7.58	0.	12.72	0.	SI
34.	34.	5.	1.	-.9553	0.	.1	41.28	7.67	0.	12.92	0.	SI

> 34.	0.	5.	1.	- .9553!	0.	.1	41.28	7.67	0.	12.91	0.	!SI
69.	35.	5.	2.	-3.1875!	- .01!	.34!	56.99	30.1	.0001	24.59	0.	!SI
> 69.	0.	5.	2.	-3.9344!	- .01!	.42!	56.99	30.1	.0001	24.59	0.	!SI
104.	35.	5.	2.	129.8192!	- .35!	14.43!	45.62	11.96	.0041	11.98	.005!	!SI
> 104.	0.	5.	2.	129.8192!	- .35!	14.43!	45.62	11.96	.0041	11.99	.005!	!SI
141.	37.	5.	3.	268.45	- .71!	28.17!	45.62	11.8	.008	11.68	.009!	!SI
177.	73.	5.	3.	405.372	-1.02!	38.65!	56.23	14.58	.011	11.51	.013!	!SI
214.	110.	5.	3.	540.5862!	-1.36!	51.51!	56.23	14.38	.0147	11.2	.016!	!SI
> 214.	0.	5.	3.	540.5862!	-1.36!	51.51!	56.23	14.38	.0147	11.21	.016!	!SI
232.	18.	5.	3.	603.4933!	-1.52!	57.51!	56.23	14.37	.017	11.18	.019!	!SI
249.	35.	5.	3.	665.8942!	-1.67!	63.45!	56.23	14.35	.0199	11.15	.022!	!SI
> 249.	0.	5.	3.	665.8942!	-1.67!	63.45!	56.23	14.35	.0199	11.15	.022!	!SI
284.	35.	5.	3.	767.5207!	-2.02!	73.67!	56.23	14.35	.0247	11.15	.028!	!SI
> 284.	0.	5.	3.	767.5207!	-2.02!	73.67!	56.23	14.42	.0247	11.28	.028!	!SI
724.	440.	5.	5.	1870.1781!	-4.88!	139.19!	77.47	17.86	.0566	8.83	.05!	!SI
> 724.	0.	5.	5.	1870.1781!	-4.88!	139.19!	77.47	17.87	.0566	8.84	.05!	!SI
759.	35.	5.	5.	1944.3806!	-5.07!	144.7!	77.47	17.84	.0593	8.79	.052!	!SI
> 759.	0.	5.	5.	1944.3806!	-5.07!	144.7!	77.47	17.84	.0593	8.79	.052!	!SI
794.	35.	5.	5.	1988.5033!	-5.19!	147.99!	77.47	17.84	.0608	8.79	.053!	!SI
> 794.	0.	5.	5.	1988.5033!	-5.19!	148.01!	77.47	17.9	.0608	8.91	.054!	!SI
850.	56.	5.	5.	2047.3942!	-5.34!	152.33!	77.47	17.73	.0629	8.62	.054!	!SI
1234.	440.	5.	5.	2370.4001!	-6.17!	175.9!	77.47	16.56	.0746	6.65	.05!	!SI
>1234.	0.	5.	5.	2370.4001!	-6.17!	175.9!	77.47	16.57	.0746	6.66	.05!	!SI
1252.	18.	5.	5.	2378.9761!	-6.19!	176.53!	77.47	16.55	.0749	6.63	.05!	!SI
1269.	35.	5.	5.	2387.0459!	-6.21!	177.12!	77.47	16.54	.0752	6.6	.05!	!SI
>1269.	0.	5.	5.	2387.0459!	-6.21!	177.12!	77.47	16.54	.0752	6.6	.05!	!SI
1304.	35.	5.	5.	2370.4109!	-6.17!	175.89!	77.47	16.54	.0746	6.6	.049!	!SI
>1304.	0.	5.	5.	2370.4109!	-6.17!	175.92!	77.47	16.6	.0746	6.72	.05!	!SI
1744.	440.	5.	5.	1988.4956!	-5.16!	147.12!	77.47	15.26	.0613	5.48	.034!	!SI
>1744.	0.	5.	5.	1988.4956!	-5.16!	147.13!	77.47	15.27	.0613	5.49	.034!	!SI
1779.	35.	5.	5.	1944.3836!	-5.05!	143.85!	77.47	15.24	.0598	5.44	.033!	!SI
>1779.	0.	5.	5.	1944.3836!	-5.05!	143.85!	77.47	15.24	.0598	5.44	.033!	!SI
1814.	35.	5.	5.	1870.187!	-4.85!	138.36!	77.47	15.24	.0572	5.44	.031!	!SI
>1814.	0.	5.	5.	1870.187!	-4.85!	138.38!	77.47	15.3	.0572	5.56	.032!	!SI
2004.	190.	5.	5.	1427.4096!	-3.83!	114.12!	66.85	11.92	.0461	14.63	.067!	!SI
2254.	440.	5.	3.	767.5092!	-2.	72.85!	56.23	9.18	.0269	14.24	.038!	!SI
>2254.	0.	5.	3.	767.5092!	-2.	72.85!	56.23	9.18	.0269	14.25	.038!	!SI
2289.	35.	5.	3.	665.9	-1.67!	62.81!	56.23	9.15	.0221	14.23	.032!	!SI
>2289.	0.	5.	3.	665.9	-1.67!	62.81!	56.23	9.15	.0221	14.23	.032!	!SI
2324.	35.	5.	3.	540.6028!	-1.35!	50.99!	56.23	9.15	.0165	14.23	.023!	!SI
>2324.	0.	5.	3.	540.6028!	-1.35!	50.99!	56.23	9.16	.0165	14.24	.023!	!SI
2434.	110.	5.	2.	129.8085!	- .34!	14.26!	45.62	5.78	.0041	13.33	.005!	!SI
>2434.	0.	5.	2.	129.8085!	- .34!	14.26!	45.62	5.79	.0041	13.33	.005!	!SI
2469.	35.	5.	2.	-3.9344!	- .01!	.42!	56.99	36.13	.0001	32.47	0.	!SI
>2469.	0.	5.	2.	-3.1876!	- .01!	.34!	56.99	36.13	.0001	32.47	0.	!SI
2504.	35.	5.	1.	- .9553!	0.	.1	56.99	25.75	0.	26.08	0.	!SI
>2504.	0.	5.	1.	- .9553!	0.	.1	56.99	25.75	0.	26.07	0.	!SI
2521.	17.	5.	1.	- .2388!	0.	.03	56.99	25.84	0.	26.13	0.	!SI

## VERIFICA SEZIONI TRAVI NORD

### PARAMETRI GENERALI

Tipo verifica : stati limite - pressoflessione deviata.  
 Unità di misura generiche: kN; cm; kNm; N/mm2; d in mm; deformazioni\*1000.  
 ferri : diametri in mm; aree in cm2.  
 Simboli : Vert. = contorno\_vertice del CLS; d = diametro;  
 S = Sigma (tensioni sui materiali);  
 D = Deformazioni x 1000 (epsilon);  
 Ve = colonna che indica se la verifica e' soddisfatta;

### CARATTERISTICHE MATERIALI

Calcestruzzo: Rck = 30. ; fck = 24.9 ; fcd = 14.11 (.35%)  
 Acciaio : Tipo= B450C ; ftk = 540. ; fyk = 450. ; ftd = 469.565 (6.75%)  
 Fibre : Tipo= FRCM in PBO; Samm= 1036.3 ; n=12.45 ; (valutate alle TA)

### CARATTERISTICHE SEZIONE

L'asse Z e' rivolto verso destra, l'asse Y e' rivolto verso l'alto.

Tipo sezione: a z sinistra

Cls:		Acciaio lento:		ferro		Z	Y	d[mm]	Af[cm2]
1- 1	0.	0.	1	3.	17.	20.	3.1416		
1- 2	0.	20.	2	3.	9.9	20.	3.1416		
1- 3	90.	20.	3	3.	2.8	20.	3.1416		
1- 4	140.7	-145.	4	45.4	17.	8.	.5027		
1- 5	185.	-145.	5	41.	.3	8.	.5027		
1- 6	185.	-170.	6	93.7	-2.2	8.	.5027		
1- 7	127.5	-170.	7	79.1	-2.1	8.	.5027		
1- 8	76.8	-5.	8	87.8	17.	20.	3.1416		
			9	105.4	-40.4	8.	.5027		
			10	91.	-40.9	8.	.5027		
			11	116.2	-75.4	8.	.5027		
			12	102.2	-77.5	8.	.5027		
			13	130.2	-121.1	8.	.5027		
			14	115.3	-120.1	8.	.5027		
			15	182.	-148.	16.	2.0106		
			16	182.	-167.	16.	2.0106		
			17	144.1	-166.3	26.	5.3093		
			18	139.3	-166.6	26.	5.3093		
			19	134.5	-166.8	26.	5.3093		
			20	129.7	-167.	26.	5.3093		
			21	142.7	-161.6	26.	5.3093		
			22	137.8	-161.6	26.	5.3093		
			23	132.9	-161.5	26.	5.3093		
			24	128.	-161.4	26.	5.3093		
			25	141.	-156.1	26.	5.3093		
			26	136.	-155.8	26.	5.3093		
			27	131.	-155.4	26.	5.3093		
			28	126.	-155.1	26.	5.3093		
			29	133.2	-148.	26.	5.3093		
			30	128.5	-148.	26.	5.3093		

Fibre (fase di applicazione 1 dopo carichi permanenti):

Fib.	fas	Z1	Y1	Z2	Y2	spe[mm]	A [cm2]
1	1	0.	0.	0.	0.	10.	20.
2	1	0.	20.	0.	20.	10.	90.
3	1	90.	20.	90.	20.	10.	172.614
4	1	140.7	-145.	140.7	-145.	10.	44.3
5	1	185.	-145.	185.	-145.	10.	25.
6	1	185.	-170.	185.	-170.	10.	57.5
7	1	127.5	-170.	127.5	-170.	10.	172.614
8	1	76.8	-5.	76.8	-5.	10.	76.9626

### CASI DI CARICO

Nome	Descrizione	Tipo	Ses
1	SLU	SLU (statico)	1
2	SLU VENTOY	SLU (statico)	2
6	SLU con SISMAY PRINC	SLU (sismico)	32
7	SLU con SISMAY PRINC	SLU (sismico)	32
8	SLU con SISMAY PRINC	SLU (sismico)	32
13	SLU Solo Perm.	SLU (statico)	1
16	Rara	RARA	1
17	Rara VentoY	RARA	2
18	Frequente	FREQUENTE	1
19	Frequente VentoY	FREQUENTE	2
20	Quasi Perm	QUASI PERMAN.	1

Sforzi normali applicati in z= 99.8 ; y= -66.94 (baricentro CLS)

Convenzioni: N + trazione; Mz + fib.inferiori tese; My + fib.sinistra tese.

## Descrizione : Trave Tipo NORD

### SOLLECITAZIONI AGENTI

N.	N	MZ	My	Descrizione	So1: ultima/agente =fs (>=1 OK)
1	-3.83	-5.3947	44.9331	Caso 2.1-A3174- P3	
2	-53.14	2712.3648	-119.7901	Caso 2.1-A3166- P1	Mz+:2712.3648/3820.2159= .71
3	-53.08	2712.3648	-119.8602	Caso 2.1-A3165- P3	Mz+:2712.3648/3820.1815= .71
4	6.4	2204.4824	97.3275	Caso 2.2-A3164- P3	Mz+:2204.4824/3790.9672= .5815
5	-53.3	2585.6437	-58.7463	Caso 2.1-A3164- P1	Mz+:2585.6437/3171.0279= .8154
6	6.93	666.2341	-19.0351	Caso 8.7-A3160- P1	

### RISULTATI

Piani di equilibrio (eps= muz \* y +muy \* z + lam):

Sol.	muz	muy	lambda
1.	-.00000087634	-.00000194468	.00015285592
2.	-.0000564621	-.00009719528	.00637906852
3.	-.00005644736	-.00009716506	.00637684213
4.	-.00004988933	-.00009308107	.00648738495
5.	-.00005532392	-.00009743458	.00650194724
6.	-.00000863337	-.00001365216	.00086176651

### DEFORMAZIONI SUI MATERIALI

Cls Acciaio lento

sol	vert.	D	cls	S	cls	Ve	ferro	D	ferri	S	ferri	Ve
1	1- 1		.1529		0.	si	1		.1321		26.42	si
1	1- 2		.1353		0.	si	2		.1383		27.67	si
1	1- 3		-.0397		-.55	si	3		.1446		28.91	si
1	1- 4		-.0063		0.	si	4		.0497		9.94	si
1	1- 5		-.0798		-1.1	si	5		.0728		14.55	si
1	1- 6		-.0579		-.81	si	6		-.0274		-5.48	si
1	1- 7		.0539		0.	si	7		.001		.2	si
1	1- 8		.0079		0.	si	8		-.0328		-6.55	si
1							9		-.0167		-3.35	si
1							10		.0118		2.36	si
1							11		-.007		-1.4	si
1							12		.022		4.4	si
1							13		.0058		1.15	si
1							14		.0339		6.77	si
1							15		-.0714		-14.28	si
1							16		-.0547		-10.95	si
1							17		.0184		3.67	si
1							18		.0279		5.58	si
1							19		.0374		7.48	si
1							20		.0469		9.39	si
1							21		.017		3.41	si
1							22		.0265		5.3	si
1							23		.0359		7.19	si
1							24		.0454		9.08	si
1							25		.0155		3.1	si
1							26		.0249		4.98	si
1							27		.0342		6.85	si
1							28		.0436		8.72	si
1							29		.0235		4.7	si
1							30		.0326		6.52	si
2	1- 1		6.3791		0.	si	1		5.1276		395.09	si
2	1- 2		5.2498		0.	si	2		5.5282		395.57	si
2	1- 3		-3.4977		-14.11	si	3		5.9288		396.05	si
2	1- 4		.8907		0.	si	4		1.0073		201.46	si
2	1- 5		-3.4151		-14.11	si	5		2.3722		391.8	si
2	1- 6		-2.0035		-14.11	si	6		-2.6016		-392.07	si
2	1- 7		3.5852		0.	si	7		-1.1843		-236.86	si
2	1- 8		-.8032		-9.06	si	8		-3.1129		-392.69	si
2							9		-1.5854		-317.08	si
2							10		-.1522		-30.45	si
2							11		-.6566		-131.31	si
2							12		.821		164.21	si
2							13		.5607		112.13	si
2							14		1.9527		390.55	si
2							15		-2.9541		-392.5	si
2							16		-1.8813		-376.26	si
2							17		1.7629		352.59	si
2							18		2.2421		391.65	si
2							19		2.7212		392.22	si
2							20		3.2004		392.79	si
2							21		1.6376		327.52	si
2							22		2.1093		391.49	si
2							23		2.5809		392.05	si
2							24		3.0525		392.61	si
2							25		1.4909		298.18	si
2							26		1.9549		390.98	si
2							27		2.4189		391.86	si
2							28		2.883		392.41	si
2							29		1.7867		357.34	si
2							30		2.2422		391.65	si
3	1- 1		6.3768		0.	si	1		5.1257		395.09	si
3	1- 2		5.2479		0.	si	2		5.5262		395.57	si
3	1- 3		-3.497		-14.11	si	3		5.9267		396.04	si
3	1- 4		.8906		0.	si	4		1.0067		201.34	si
3	1- 5		-3.4138		-14.11	si	5		2.3712		391.8	si
3	1- 6		-2.0026		-14.11	si	6		-2.601		-392.07	si
3	1- 7		3.5843		0.	si	7		-1.1842		-236.83	si
3	1- 8		-.8032		-9.06	si	8		-3.1122		-392.68	si
3							9		-1.585		-317.01	si
3							10		-.1523		-30.47	si
3							11		-.6564		-131.28	si
3							12		.8208		164.15	si
3							13		.5606		112.12	si
3							14		1.9522		390.44	si
3							15		-2.953		-392.49	si
3							16		-1.8805		-376.1	si
3							17		1.7626		352.52	si
3							18		2.2416		391.64	si
3							19		2.7206		392.22	si
3							20		3.1996		392.79	si
3							21		1.6373		327.46	si
3							22		2.1088		391.49	si
3							23		2.5803		392.05	si
3							24		3.0517		392.61	si
3							25		1.4906		298.12	si
3							26		1.9545		390.9	si
3							27		2.4184		391.86	si
3							28		2.8823		392.41	si
3							29		1.7863		357.27	si
3							30		2.2416		391.64	si
4	1- 1		6.4874		0.	si	1		5.36		395.37	si
4	1- 2		5.4896		0.	si	2		5.7139		395.79	si
4	1- 3		-2.8877		-14.11	si	3		6.0679		396.21	si
4	1- 4		.6248		0.	si	4		1.4141		282.83	si
4	1- 5		-3.4987		-14.11	si	5		2.6516		392.13	si
4	1- 6		-2.2514		-14.11	si	6		-2.1224		-391.5	si

4	1- 7	3.1007	0.	si	7	-.7648	-152.96	si
4	1- 8	-.4118	-5.21	si	8	-2.5317	-391.99	si
4					9	-1.3091	-261.82	si
4					10	.0613	12.25	si
4					11	-.5656	-113.13	si
4					12	.8403	168.06	si
4					13	.4086	81.73	si
4					14	1.7461	349.22	si
4					15	-3.0697	-392.63	si
4					16	-2.1219	-391.5	si
4					17	1.3709	274.19	si
4					18	1.8289	365.77	si
4					19	2.2868	391.7	si
4					20	2.7447	392.25	si
4					21	1.2706	254.12	si
4					22	1.7226	344.52	si
4					23	2.1744	391.56	si
4					24	2.6263	392.1	si
4					25	1.1532	230.64	si
4					26	1.599	319.8	si
4					27	2.0448	391.41	si
4					28	2.4907	391.94	si
4					29	1.4704	294.07	si
4					30	1.9066	381.31	si
5	1- 1	6.5019	0.	si	1	5.2691	395.26	si
5	1- 2	5.3955	0.	si	2	5.6616	395.73	si
5	1- 3	-3.3736	-14.11	si	3	6.0541	396.2	si
5	1- 4	.8149	0.	si	4	1.1387	227.74	si
5	1- 5	-3.5000	-14.11	si	5	2.4857	391.94	si
5	1- 6	-2.1184	-14.11	si	6	-2.5036	-391.96	si
5	1- 7	3.4841	0.	si	7	-1.0828	-216.56	si
5	1- 8	-.7044	-8.19	si	8	-2.9917	-392.54	si
5					9	-1.5338	-306.76	si
5					10	-.0977	-19.55	si
5					11	-.6472	-129.45	si
5					12	.8312	166.24	si
5					13	.5145	102.9	si
5					14	1.9113	382.27	si
5					15	-3.0432	-392.6	si
5					16	-1.9921	-391.35	si
5					17	1.662	332.4	si
5					18	2.142	391.53	si
5					19	2.6221	392.1	si
5					20	3.1021	392.67	si
5					21	1.5424	308.47	si
5					22	2.0153	391.37	si
5					23	2.4881	391.94	si
5					24	2.961	392.5	si
5					25	1.4024	280.47	si
5					26	1.868	373.59	si
5					27	2.3336	391.75	si
5					28	2.7992	392.31	si
5					29	1.7093	341.85	si
5					30	2.1658	391.55	si
6	1- 1	.8618	0.	si	1	.674	134.81	si
6	1- 2	.6891	0.	si	2	.7353	147.06	si
6	1- 3	-.5396	-6.59	si	3	.7965	159.31	si
6	1- 4	.1927	0.	si	4	.0953	19.06	si
6	1- 5	-.412	-5.22	si	5	.2987	59.75	si
6	1- 6	-.1962	-2.63	si	6	-.3981	-79.62	si
6	1- 7	.5888	0.	si	7	-.1991	-39.82	si
6	1- 8	-.1436	-1.95	si	8	-.4834	-96.69	si
6					9	-.2285	-45.7	si
6					10	-.0269	-5.37	si
6					11	-.0735	-14.7	si
6					12	.1356	27.11	si
6					13	.1296	25.92	si
6					14	.3244	64.88	si
6					15	-.3452	-69.04	si
6					16	-.1812	-36.23	si
6					17	.3302	66.05	si
6					18	.3977	79.54	si
6					19	.4652	93.03	si
6					20	.5326	106.52	si
6					21	.3093	61.87	si
6					22	.3755	75.11	si
6					23	.4417	88.35	si
6					24	.5079	101.59	si
6					25	.2848	56.97	si
6					26	.3498	69.96	si
6					27	.4147	82.94	si
6					28	.4797	95.93	si
6					29	.3207	64.14	si
6					30	.3847	76.94	si

Fibre (FRCM in PBO)

sol	fib	ver	D impr.	D cls	D fibra	S fibra	Ve
1	1	1	0.	.01165	.01165	4.56	si
1	1	2	0.	.00972	.00972	3.81	si
1	2	1	0.	.00972	.00972	3.81	si
1	2	2	0.	-.00638	-.00638	-2.5	si
1	3	1	0.	-.00638	-.00638	-2.5	si
1	3	2	0.	.00048	.00048	.19	si
1	4	1	0.	.00048	.00048	.19	si
1	4	2	0.	-.00745	-.00745	-2.92	si
1	5	1	0.	-.00745	-.00745	-2.92	si
1	5	2	0.	-.00503	-.00503	-1.97	si
1	6	1	0.	-.00503	-.00503	-1.97	si

1	6	2	0.	.00525	.00525	2.06	si
1	7	1	0.	.00525	.00525	2.06	si
1	7	2	0.	-.00161	-.00161	-.63	si
1	8	1	0.	-.00161	-.00161	-.63	si
1	8	2	0.	.01165	.01165	4.56	si
2	1	1	0.	.26166	.26166	102.45	si
2	1	2	0.	.17681	.17681	69.22	si
2	2	1	0.	.17681	.17681	69.22	si
2	2	2	0.	-.31005	-.31005	-121.39	si
2	3	1	0.	-.31005	-.31005	-121.39	si
2	3	2	0.	.11574	.11574	45.32	si
2	4	1	0.	.11574	.11574	45.32	si
2	4	2	0.	-.1239	-.1239	-48.51	si
2	5	1	0.	-.1239	-.1239	-48.51	si
2	5	2	0.	-.01783	-.01783	-6.98	si
2	6	1	0.	-.01783	-.01783	-6.98	si
2	6	2	0.	.29322	.29322	114.8	si
2	7	1	0.	.29322	.29322	114.8	si
2	7	2	0.	-.13257	-.13257	-51.9	si
2	8	1	0.	-.13257	-.13257	-51.9	si
2	8	2	0.	.26166	.26166	102.45	si
3	1	1	0.	.26166	.26166	102.44	si
3	1	2	0.	.1768	.1768	69.22	si
3	2	1	0.	.1768	.1768	69.22	si
3	2	2	0.	-.31005	-.31005	-121.39	si
3	3	1	0.	-.31005	-.31005	-121.39	si
3	3	2	0.	.11575	.11575	45.32	si
3	4	1	0.	.11575	.11575	45.32	si
3	4	2	0.	-.12389	-.12389	-48.51	si
3	5	1	0.	-.12389	-.12389	-48.51	si
3	5	2	0.	-.01782	-.01782	-6.98	si
3	6	1	0.	-.01782	-.01782	-6.98	si
3	6	2	0.	.29322	.29322	114.8	si
3	7	1	0.	.29322	.29322	114.8	si
3	7	2	0.	-.13257	-.13257	-51.9	si
3	8	1	0.	-.13257	-.13257	-51.9	si
3	8	2	0.	.26166	.26166	102.44	si
4	1	1	0.	.29327	.29327	114.82	si
4	1	2	0.	.20723	.20723	81.13	si
4	2	1	0.	.20723	.20723	81.13	si
4	2	2	0.	-.31004	-.31004	-121.39	si
4	3	1	0.	-.31004	-.31004	-121.39	si
4	3	2	0.	.10844	.10844	42.46	si
4	4	1	0.	.10844	.10844	42.46	si
4	4	2	0.	-.14617	-.14617	-57.23	si
4	5	1	0.	-.14617	-.14617	-57.23	si
4	5	2	0.	-.03861	-.03861	-15.12	si
4	6	1	0.	-.03861	-.03861	-15.12	si
4	6	2	0.	.29187	.29187	114.27	si
4	7	1	0.	.29187	.29187	114.27	si
4	7	2	0.	-.12662	-.12662	-49.57	si
4	8	1	0.	-.12662	-.12662	-49.57	si
4	8	2	0.	.29327	.29327	114.82	si
5	1	1	0.	.2697	.2697	105.59	si
5	1	2	0.	.18463	.18463	72.29	si
5	2	1	0.	.18463	.18463	72.29	si
5	2	2	0.	-.31004	-.31004	-121.39	si
5	3	1	0.	-.31004	-.31004	-121.39	si
5	3	2	0.	.11309	.11309	44.28	si
5	4	1	0.	.11309	.11309	44.28	si
5	4	2	0.	-.1304	-.1304	-51.05	si
5	5	1	0.	-.1304	-.1304	-51.05	si
5	5	2	0.	-.02406	-.02406	-9.42	si
5	6	1	0.	-.02406	-.02406	-9.42	si
5	6	2	0.	.29198	.29198	114.32	si
5	7	1	0.	.29198	.29198	114.32	si
5	7	2	0.	-.13116	-.13116	-51.35	si
5	8	1	0.	-.13116	-.13116	-51.35	si
5	8	2	0.	.2697	.2697	105.59	si
6	1	1	0.	.07364	.07364	28.83	si
6	1	2	0.	.05047	.05047	19.76	si
6	2	1	0.	.05047	.05047	19.76	si
6	2	2	0.	-.08393	-.08393	-32.86	si
6	3	1	0.	-.08393	-.08393	-32.86	si
6	3	2	0.	.03147	.03147	12.32	si
6	4	1	0.	.03147	.03147	12.32	si
6	4	2	0.	-.03469	-.03469	-13.58	si
6	5	1	0.	-.03469	-.03469	-13.58	si
6	5	2	0.	-.00573	-.00573	-2.24	si
6	6	1	0.	-.00573	-.00573	-2.24	si
6	6	2	0.	.08014	.08014	31.38	si
6	7	1	0.	.08014	.08014	31.38	si
6	7	2	0.	-.03526	-.03526	-13.81	si
6	8	1	0.	-.03526	-.03526	-13.81	si
6	8	2	0.	.07364	.07364	28.83	si

## VERIFICA SEZIONI TRAVI SUD

### PARAMETRI GENERALI

Tipo verifica : stati limite - pressoflessione deviata.  
 Unità di misura generiche: kN; cm; kNm; N/mm<sup>2</sup>; d in mm; deformazioni\*1000.  
 ferri : diametri in mm; aree in cm<sup>2</sup>.  
 Simboli : Vert. = contorno\_vertice del CLS; d = diametro;  
 S = Sigma (tensioni sui materiali);  
 D = Deformazioni x 1000 (epsilon);  
 Ve = colonna che indica se la verifica e' soddisfatta;

### CARATTERISTICHE MATERIALI

Calcestruzzo: Rck = 30. ; fck = 24.9 ; fcd = 14.11 (.35%)  
 Acciaio : Tipo= B450C ; ftk = 540. ; fyk = 450. ; ftd = 469.565 (6.75%)  
 Fibre : Tipo= FRCM in PBO; Samm= 1036.3 ; n=12.45 ; (valutate alle TA)

### CARATTERISTICHE SEZIONE

L'asse Z e' rivolto verso destra, l'asse Y e' rivolto verso l'alto.

Tipo sezione: a z destra

CLS:

vert.		Z	Y	Acciaio lento:		d[mm]	Af[cm2]	
ferro		Z	Y					
1- 1	0.	0.		1	182.	187.	20.	3.1416
1- 2	0.	25.		2	182.	179.9	20.	3.1416
1- 3	44.3	25.		3	182.	172.8	20.	3.1416
1- 4	95.	190.		4	139.6	187.	8.	.5027
1- 5	185.	190.		5	144.	170.3	8.	.5027
1- 6	185.	170.		6	91.7	168.9	8.	.5027
1- 7	108.2	165.		7	105.9	167.9	8.	.5027
1- 8	57.5	0.		8	95.7	134.5	8.	.5027
				9	81.2	134.9	8.	.5027
				10	85.6	101.8	8.	.5027
				11	70.8	101.	8.	.5027
				12	72.3	58.3	8.	.5027
				13	57.4	57.5	8.	.5027
				14	97.2	187.	20.	3.1416
				15	3.	22.	16.	2.0106
				16	3.	3.	16.	2.0106
				17	39.5	3.	26.	5.3093
				18	44.8	3.	26.	5.3093
				19	50.	3.	26.	5.3093
				20	55.3	3.	26.	5.3093
				21	39.4	9.1	26.	5.3093
				22	45.3	9.	26.	5.3093
				23	51.2	9.	26.	5.3093
				24	57.1	8.9	26.	5.3093
				25	41.1	14.4	26.	5.3093
				26	47.1	14.8	26.	5.3093
				27	53.1	15.2	26.	5.3093
				28	59.2	15.6	26.	5.3093
				29	50.	22.	26.	5.3093
				30	55.	22.	26.	5.3093

Fibre (fase di applicazione 1 dopo carichi permanenti):

Fib.	fas	Z1	Y1	Z2	Y2	spe[mm]	A [cm2]
1	1	0.	0.	0.	0.	10.	25.
2	1	0.	25.	0.	25.	10.	44.3
3	1	44.3	25.	44.3	25.	10.	172.614
4	1	95.	190.	95.	190.	10.	90.
5	1	185.	190.	185.	190.	10.	20.
6	1	185.	170.	185.	170.	10.	76.9626
7	1	108.2	165.	108.2	165.	10.	172.614
8	1	57.5	0.	57.5	0.	10.	57.5

### CASI DI CARICO

Nome	Descrizione	Tipo	Ses
1	SLU	SLU (statico)	1
2	SLU VENTOY	SLU (statico)	2
6	SLU con SISMAY PRINC	SLU (sismico)	32
7	SLU con SISMAY PRINC	SLU (sismico)	32
8	SLU con SISMAY PRINC	SLU (sismico)	32
13	SLU Solo Perm.	SLU (statico)	1
16	Rara	RARA	1
17	Rara VentoY	RARA	2
18	Frequente	FREQUENTE	1
19	Frequente VentoY	FREQUENTE	2
20	Quasi Perm	QUASI PERMAN.	1

Sforzi normali applicati in z= 85.2 ; y= 103.06 (baricentro CLS)

Convenzioni: N + trazione; Mz + fib.inferiori tese; My + fib.sinistra tese.

## Descrizione : Trave Tipo SUD

### SOLLECITAZIONI AGENTI

N.	N	MZ	My	Descrizione	So1: ultima/agente =fs (>=1 OK)
1	-6.1	-5.4382	-69.7081	Caso 2.2-A4174- P3	
2	-55.27	2588.9311	101.4335	Caso 2.2-A4166- P1	Mz+:2588.9311/3819.0782= .6779
3	4.83	3819.078	2577.776	Caso 2.1-A4165- P3	My+:2577.776/-101.63=-25.36
4	-55.14	3819.0214	2578.3703	Caso 2.2-A4165- P3	My+:2578.3703/101.6268=25.371
5	-55.36	2497.1488	60.4572	Caso 2.2-A4164- P1	Mz+:2497.1488/3170.0868= .7877
6	6.92	665.9932	3.3701	Caso 8.29-A4160- P1	

### RISULTATI

Piani di equilibrio (eps= muz \* y +muy \* z + lam):

Sol.	muz	muy	lambda
1.	-.00000144936	.00000315569	-.0000908356
2.	-.00005771627	.0001003316	-.00205719586
3.	-.00000736087	-.00004473163	.00352536277
4.	-.00000705526	-.00004426073	.0034879496
5.	-.00005620693	.00009897883	-.0020913521
6.	-.00000930431	.00001489484	-.00021962837

### DEFORMAZIONI SUI MATERIALI

CLS

Acciaio lento



sol	vert.	D	cls	S	cls	Ve	ferro	D	ferri	S	ferri	Ve
1	1- 1		-.0908		-1.25	si	1		.2125		42.49	si
1	1- 2		-.1271		-1.74	si	2		.2228		44.55	si
1	1- 3		.0127		0.	si	3		.233		46.61	si
1	1- 4		-.0664		-.92	si	4		.0787		15.74	si
1	1- 5		.2176		0.	si	5		.1166		23.32	si
1	1- 6		.2466		0.	si	6		-.0464		-9.28	si
1	1- 7		.0115		0.	si	7		.0002		.04	si
1	1- 8		.0906		0.	si	8		.0162		3.24	si
1							9		-.0301		-6.02	si
1							10		.0319		6.38	si
1							11		-.0138		-2.77	si
1							12		.0527		10.55	si
1							13		.007		1.41	si
1							14		-.0551		-11.02	si
1							15		-.1133		-22.65	si
1							16		-.0857		-17.14	si
1							17		.0295		5.89	si
1							18		.0461		9.21	si
1							19		.0627		12.53	si
1							20		.0793		15.85	si
1							21		.0203		4.07	si
1							22		.039		7.81	si
1							23		.0578		11.55	si
1							24		.0765		15.29	si
1							25		.0178		3.56	si
1							26		.0363		7.26	si
1							27		.0548		10.95	si
1							28		.0732		14.65	si
1							29		.0352		7.04	si
1							30		.0507		10.14	si
2	1- 1		-2.0572		-14.11	si	1		5.4102		395.43	si
2	1- 2		-3.5000		-14.11	si	2		5.8197		395.92	si
2	1- 3		.9446		0.	si	3		6.2291		396.41	si
2	1- 4		-3.4918		-14.11	si	4		1.157		231.39	si
2	1- 5		5.5381		0.	si	5		2.5564		392.02	si
2	1- 6		6.6924		0.	si	6		-2.6105		-392.09	si
2	1- 7		-.7245		-8.37	si	7		-1.1163		-223.25	si
2	1- 8		3.7119		0.	si	8		-.2197		-43.94	si
2							9		-1.6954		-339.07	si
2							10		.6597		131.95	si
2							11		-.7845		-156.91	si
2							12		1.8286		365.72	si
2							13		.3852		77.04	si
2							14		-3.0962		-392.67	si
2							15		-3.026		-392.58	si
2							16		-1.9293		-385.87	si
2							17		1.7325		346.49	si
2							18		2.2604		391.67	si
2							19		2.7883		392.3	si
2							20		3.3163		392.93	si
2							21		1.3709		274.19	si
2							22		1.9669		391.32	si
2							23		2.5627		392.03	si
2							24		3.1586		392.74	si
2							25		1.2288		245.77	si
2							26		1.8117		362.34	si
2							27		2.3946		391.83	si
2							28		2.9776		392.52	si
2							29		1.6938		338.77	si
2							30		2.1875		391.58	si
3	1- 1		3.5254		0.	si	1		-3.2393		-392.84	si
3	1- 2		3.7094		0.	si	2		-3.2915		-392.9	si
3	1- 3		1.7278		0.	si	3		-3.3438		-392.96	si
3	1- 4		.6744		0.	si	4		-1.343		-268.61	si
3	1- 5		-3.3514		-14.11	si	5		-1.6608		-332.17	si
3	1- 6		-3.4986		-14.11	si	6		.6685		133.71	si
3	1- 7		-.1001		-1.38	si	7		.0221		4.42	si
3	1- 8		.9533		0.	si	8		.235		47.	si
3							9		.8858		177.17	si
3							10		.4437		88.75	si
3							11		1.1021		220.41	si
3							12		.7213		144.25	si
3							13		1.3798		275.97	si
3							14		.5532		110.63	si
3							15		3.5531		393.21	si
3							16		3.4133		393.04	si
3							17		1.7807		356.14	si
3							18		1.5453		309.06	si
3							19		1.3099		261.98	si
3							20		1.0745		214.91	si
3							21		1.829		365.8	si
3							22		1.565		313.01	si
3							23		1.3011		260.22	si
3							24		1.0371		207.42	si
3							25		1.7952		359.05	si
3							26		1.5282		305.64	si
3							27		1.2612		252.23	si
3							28		.9941		198.82	si
3							29		1.4488		289.77	si
3							30		1.2288		245.75	si
4	1- 1		3.4879		0.	si	1		-3.2482		-392.85	si
4	1- 2		3.6643		0.	si	2		-3.2982		-392.91	si
4	1- 3		1.7036		0.	si	3		-3.3483		-392.97	si
4	1- 4		.6237		0.	si	4		-1.3719		-274.37	si
4	1- 5		-3.3598		-14.11	si	5		-1.6825		-336.5	si
4	1- 6		-3.5000		-14.11	si	6		.6227		124.53	si

4	1- 7	-.1369	-1.87	si	7	-.0167	-3.35	si
4	1- 8	.943	0.	si	8	.2015	40.31	si
4					9	.8454	169.09	si
4					10	.4156	83.11	si
4					11	1.0671	213.42	si
4					12	.7001	140.01	si
4					13	1.3519	270.38	si
4					14	.5044	100.88	si
4					15	3.5104	393.16	si
4					16	3.3763	393.	si
4					17	1.7609	352.19	si
4					18	1.528	305.61	si
4					19	1.2951	259.03	si
4					20	1.0622	212.45	si
4					21	1.8074	361.48	si
4					22	1.5462	309.24	si
4					23	1.285	257.01	si
4					24	1.0239	204.77	si
4					25	1.7728	354.55	si
4					26	1.5084	301.69	si
4					27	1.2441	248.82	si
4					28	.9798	195.95	si
4					29	1.4283	285.65	si
4					30	1.2105	242.1	si
5	1- 1	-2.0914	-14.11	si	1	5.4121	395.43	si
5	1- 2	-3.4965	-14.11	si	2	5.8108	395.91	si
5	1- 3	.8882	0.	si	3	6.2096	396.38	si
5	1- 4	-3.3677	-14.11	si	4	1.2162	243.24	si
5	1- 5	5.5404	0.	si	5	2.5846	392.05	si
5	1- 6	6.6646	0.	si	6	-2.5137	-391.97	si
5	1- 7	-.656	-7.74	si	7	-1.0404	-208.07	si
5	1- 8	3.5999	0.	si	8	-.1803	-36.06	si
5					9	-1.6358	-327.15	si
5					10	.6634	132.68	si
5					11	-.762	-152.4	si
5					12	1.7847	356.94	si
5					13	.3602	72.03	si
5					14	-2.9796	-392.53	si
5					15	-3.031	-392.59	si
5					16	-1.963	-391.31	si
5					17	1.6494	329.88	si
5					18	2.1702	391.56	si
5					19	2.691	392.18	si
5					20	3.2119	392.8	si
5					21	1.2972	259.45	si
5					22	1.8851	377.01	si
5					23	2.4728	391.92	si
5					24	3.0606	392.62	si
5					25	1.1609	232.18	si
5					26	1.7362	347.25	si
5					27	2.3116	391.73	si
5					28	2.887	392.42	si
5					29	1.6252	325.04	si
5					30	2.1122	391.49	si
6	1- 1	-.2196	-2.93	si	1	.7513	150.27	si
6	1- 2	-.4522	-5.66	si	2	.8173	163.47	si
6	1- 3	.2076	0.	si	3	.8833	176.67	si
6	1- 4	-.5724	-6.92	si	4	.1199	23.98	si
6	1- 5	.7681	0.	si	5	.3399	67.99	si
6	1- 6	.9542	0.	si	6	-.4261	-85.22	si
6	1- 7	-.1432	-1.95	si	7	-.2035	-40.7	si
6	1- 8	.6368	0.	si	8	-.0458	-9.17	si
6					9	-.2652	-53.04	si
6					10	.1088	21.76	si
6					11	-.105	-21.01	si
6					12	.3143	62.86	si
6					13	.1006	20.13	si
6					14	-.5115	-102.3	si
6					15	-.3796	-75.93	si
6					16	-.2029	-40.57	si
6					17	.3408	68.15	si
6					18	.4191	83.83	si
6					19	.4975	99.5	si
6					20	.5759	115.18	si
6					21	.2826	56.51	si
6					22	.3711	74.22	si
6					23	.4596	91.93	si
6					24	.5482	109.63	si
6					25	.2576	51.52	si
6					26	.3438	68.77	si
6					27	.4301	86.02	si
6					28	.5163	103.27	si
6					29	.321	64.21	si
6					30	.3943	78.87	si

Fibre (FRCM in PBO)

sol	fib	ver	D impr.	D cls	D fibra	S fibra	Ve
1	1	1	0.	-.0078	-.0078	-3.06	si
1	1	2	0.	-.01172	-.01172	-4.59	si
1	2	1	0.	-.01172	-.01172	-4.59	si
1	2	2	0.	.00095	.00095	.37	si
1	3	1	0.	.00095	.00095	.37	si
1	3	2	0.	-.01037	-.01037	-4.06	si
1	4	1	0.	-.01037	-.01037	-4.06	si
1	4	2	0.	.01538	.01538	6.02	si
1	5	1	0.	.01538	.01538	6.02	si
1	5	2	0.	.01851	.01851	7.25	si
1	6	1	0.	.01851	.01851	7.25	si

1	6	2	0.	-.00268	-.00268	-1.05	si
1	7	1	0.	-.00268	-.00268	-1.05	si
1	7	2	0.	.00864	.00864	3.38	si
1	8	1	0.	.00864	.00864	3.38	si
1	8	2	0.	-.0078	-.0078	-3.06	si
2	1	1	0.	-.03488	-.03488	-13.65	si
2	1	2	0.	-.2029	-.2029	-79.44	si
2	2	1	0.	-.2029	-.2029	-79.44	si
2	2	2	0.	.18118	.18118	70.94	si
2	3	1	0.	.18118	.18118	70.94	si
2	3	2	0.	-.48817	-.48817	-191.13	si
2	4	1	0.	-.48817	-.48817	-191.13	si
2	4	2	0.	.29212	.29212	114.37	si
2	5	1	0.	.29212	.29212	114.37	si
2	5	2	0.	.42654	.42654	167.	si
2	6	1	0.	.42654	.42654	167.	si
2	6	2	0.	-.20571	-.20571	-80.54	si
2	7	1	0.	-.20571	-.20571	-80.54	si
2	7	2	0.	.46364	.46364	181.52	si
2	8	1	0.	.46364	.46364	181.52	si
2	8	2	0.	-.03488	-.03488	-13.65	si
3	1	1	0.	-.05692	-.05692	-22.28	si
3	1	2	0.	-.2371	-.2371	-92.83	si
3	2	1	0.	-.2371	-.2371	-92.83	si
3	2	2	0.	.18592	.18592	72.79	si
3	3	1	0.	.18592	.18592	72.79	si
3	3	2	0.	-.51914	-.51914	-203.25	si
3	4	1	0.	-.51914	-.51914	-203.25	si
3	4	2	0.	.34026	.34026	133.22	si
3	5	1	0.	.34026	.34026	133.22	si
3	5	2	0.	.4844	.4844	189.65	si
3	6	1	0.	.4844	.4844	189.65	si
3	6	2	0.	-.21292	-.21292	-83.36	si
3	7	1	0.	-.21292	-.21292	-83.36	si
3	7	2	0.	.49215	.49215	192.68	si
3	8	1	0.	.49215	.49215	192.68	si
3	8	2	0.	-.05692	-.05692	-22.28	si
4	1	1	0.	-.03485	-.03485	-13.64	si
4	1	2	0.	-.20285	-.20285	-79.42	si
4	2	1	0.	-.20285	-.20285	-79.42	si
4	2	2	0.	.18118	.18118	70.94	si
4	3	1	0.	.18118	.18118	70.94	si
4	3	2	0.	-.48813	-.48813	-191.11	si
4	4	1	0.	-.48813	-.48813	-191.11	si
4	4	2	0.	.29208	.29208	114.35	si
4	5	1	0.	.29208	.29208	114.35	si
4	5	2	0.	.42648	.42648	166.97	si
4	6	1	0.	.42648	.42648	166.97	si
4	6	2	0.	-.2057	-.2057	-80.53	si
4	7	1	0.	-.2057	-.2057	-80.53	si
4	7	2	0.	.46362	.46362	181.51	si
4	8	1	0.	.46362	.46362	181.51	si
4	8	2	0.	-.03485	-.03485	-13.64	si
5	1	1	0.	-.03192	-.03192	-12.5	si
5	1	2	0.	-.1728	-.1728	-67.65	si
5	2	1	0.	-.1728	-.1728	-67.65	si
5	2	2	0.	.15052	.15052	58.93	si
5	3	1	0.	.15052	.15052	58.93	si
5	3	2	0.	-.40921	-.40921	-160.21	si
5	4	1	0.	-.40921	-.40921	-160.21	si
5	4	2	0.	.24765	.24765	96.96	si
5	5	1	0.	.24765	.24765	96.96	si
5	5	2	0.	.36034	.36034	141.08	si
5	6	1	0.	.36034	.36034	141.08	si
5	6	2	0.	-.172	-.172	-67.34	si
5	7	1	0.	-.172	-.172	-67.34	si
5	7	2	0.	.38774	.38774	151.81	si
5	8	1	0.	.38774	.38774	151.81	si
5	8	2	0.	-.03192	-.03192	-12.5	si
6	1	1	0.	-.00735	-.00735	-2.88	si
6	1	2	0.	-.03752	-.03752	-14.69	si
6	2	1	0.	-.03752	-.03752	-14.69	si
6	2	2	0.	.03224	.03224	12.62	si
6	3	1	0.	.03224	.03224	12.62	si
6	3	2	0.	-.08704	-.08704	-34.08	si
6	4	1	0.	-.08704	-.08704	-34.08	si
6	4	2	0.	.05468	.05468	21.41	si
6	5	1	0.	.05468	.05468	21.41	si
6	5	2	0.	.07881	.07881	30.86	si
6	6	1	0.	.07881	.07881	30.86	si
6	6	2	0.	-.03608	-.03608	-14.13	si
6	7	1	0.	-.03608	-.03608	-14.13	si
6	7	2	0.	.08319	.08319	32.57	si
6	8	1	0.	.08319	.08319	32.57	si
6	8	2	0.	-.00735	-.00735	-2.88	si

## ***DIAGRAMMI SFORZI-DEFORMAZIONI DELLE SEZIONI:***

N.B. Per ogni sella o trave è rappresentata la sezione soggetta al sestetto del caso di carico che induce la massima combinazione sforzi-deformazioni.

# Ripristino Ponte-Tubo

## VERIFICA SELLA DI TESTA

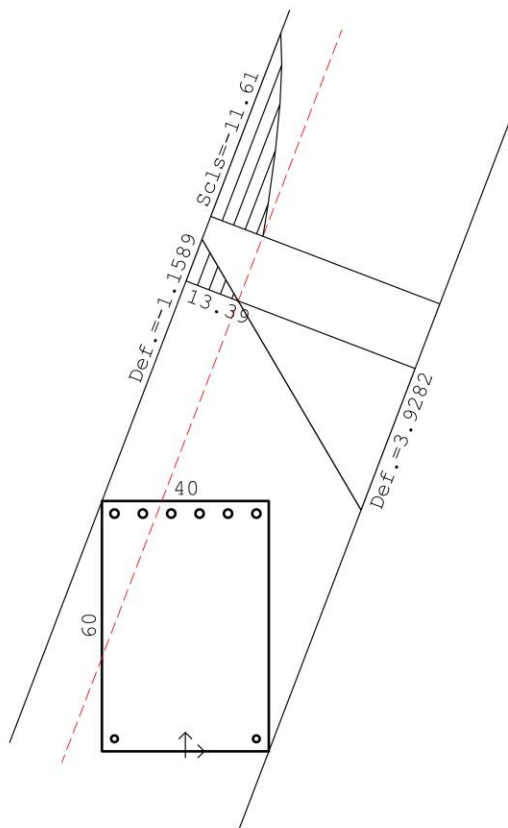
DESCRIZIONI  
 Tipo sezione : RETTANGOLARE  
 Tipo verifica: stato limite ultimo

CARATTERISTICHE DEI MATERIALI  
 Calcestruzzo: Rck= 30.  
 fck= 24.9  
 fcd= 14.11 (.35%)  
 Acciaio lento: Tipo= B450C  
 fyk= 450.  
 ftk= 540.  
 ftd= 469.57 (6.75%)

PIANO DI EQUILIBRIO:  
 eps= muz \* y +muy \* z + lam  
 muz=-3.08926800175044E-05  
 muy= 8.0838419815731E-05  
 lam= 2.3114518985812E-03

SOLLECITAZIONI AGENTI:  
 Nd in z= 0. ; y= 30. (baricentro CLS)  
 Nd = 195.26  
 Mdz= -.0763  
 Mdy= -69.1916

UNITA' DI MISURA:  
 kN; cm; kNm; N/mm2.  
 Ø in mm; deformazioni\*1000.  
 SIMBOLI:  
 S=sigma (tensioni sui materiali);  
 D=deformazioni (epsilon):



TENSIONI NEL CLS:

ver	Z	Y	Dcls	Scls	Ve
1	-20.	60.	-1.15888	-11.61	si
2	20.	60.	2.07466	0.	si
3	20.	0.	3.92822	0.	si
4	-20.	0.	.69468	0.	si

TENSIONI NEI FERRI:

fer	Z	Y	Ø (mm)	Af (cm2)	D ferri	S ferri	Ve
1	17.	57.	20	3.14	1.92482	384.96	si
2	10.2	57.	20	3.14	1.37512	275.02	si
3	3.4	57.	20	3.14	.82542	165.08	si
4	-3.4	57.	20	3.14	-.27572	-55.14	si
5	-10.2	57.	20	3.14	-.27398	-54.8	si
6	-17.	57.	20	3.14	-.82368	-164.74	si
7	17.	3.	16	2.01	3.59303	393.26	si
8	-17.	3.	16	2.01	-.84452	168.9	si

% ARMAT.: tesa= .69; comp.= .26; tot.= .95

# Ripristino Ponte-Tubo

## VERIFICA SELLA I INTERMEDIA

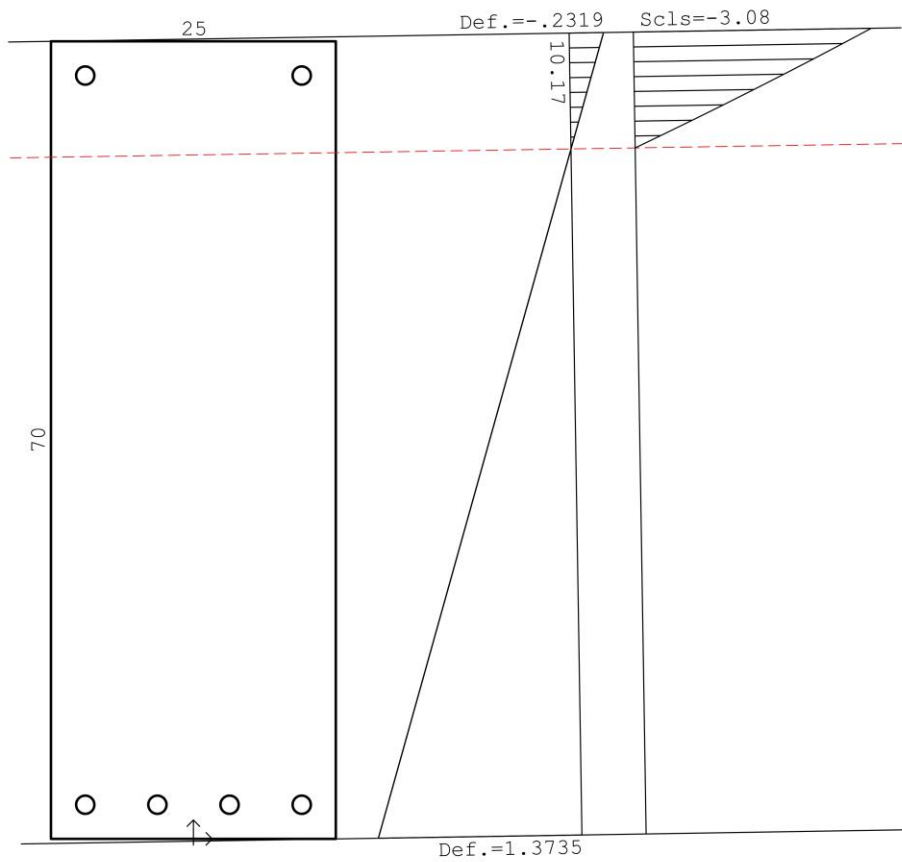
DESCRIZIONI  
 Tipo sezione : RETTANGOLARE  
 Tipo verifica: stato limite ultimo

CARATTERISTICHE DEI MATERIALI  
 Calcestruzzo: Rck= 30.  
 fck= 24.9  
 fcd= 14.11 (.35%)  
 Acciaio lento: Tipo= B450C  
 fyk= 450.  
 ftk= 540.  
 ftd= 469.57 (6.75%)

PIANO DI EQUILIBRIO:  
 eps= muz \* y +muy \* z + lam  
 muz=-2.28044261058069E-05  
 muy= 3.64706897384181E-07  
 lam= 1.36891946226862E-03

SOLLECITAZIONI AGENTI:  
 Nd in z= 0. ; y= 35. (baricentro CLS)  
 Nd = 157.92  
 Mdz= 83.207  
 Mdy= -.1189

UNITA' DI MISURA:  
 kN; cm; kNm; N/mm2.  
 Ø in mm; deformazioni\*1000.  
 SIMBOLI:  
 S=sigma (tensioni sui materiali);  
 D=deformazioni (epsilon):



TENSIONI NEL CLS:					
ver	Z	Y	Dcls	Scls	Ve
1	-12.5	70.	-.23195	-3.08	si
2	12.5	70.	-.22283	-2.97	si
3	12.5	0.	1.37348	0.	si
4	-12.5	0.	1.36436	0.	si

TENSIONI NEI FERRI:							
fer	Z	Y	Ø (mm)	Af (cm2)	D ferri	S ferri	Ve
1	9.5	67.	16	2.01	-.15551	-31.1	si
2	-9.5	67.	16	2.01	-.16244	-32.49	si
3	9.5	3.	16	2.01	1.30397	260.79	si
4	3.2	3.	16	2.01	1.30166	260.33	si
5	-3.2	3.	16	2.01	1.29935	259.87	si
6	-9.5	3.	16	2.01	1.29704	259.41	si

% ARMAT.: tesa= .46; comp.= .23; tot.= .69

# Ripristino Ponte-Tubo

## VERIFICA SELLA II INTERMEDIA

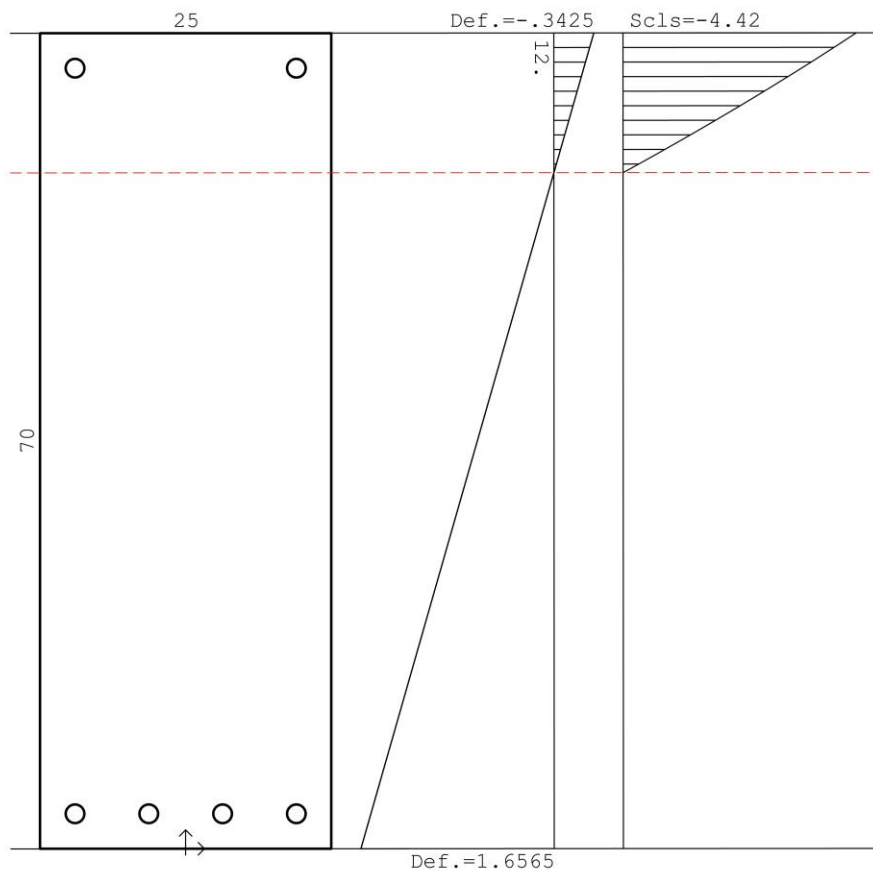
DESCRIZIONI  
 Tipo sezione : RETTANGOLARE  
 Tipo verifica: stato limite ultimo

CARATTERISTICHE DEI MATERIALI  
 Calcestruzzo: Rck= 30.  
 fck= 24.9  
 fcd= 14.11 (.35%)  
 Acciaio lento: Tipo= B450C  
 fyk= 450.  
 ftk= 540.  
 ftd= 469.57 (6.75%)

PIANO DI EQUILIBRIO:  
 eps= muz \* y +muy \* z + lam  
 muz=-2.85520938653072E-05  
 muy= 1.60722809011064E-08  
 lam= 1.65633944541267E-03

SOLLECITAZIONI AGENTI:  
 Nd in z= 0. ; y= 35. (baricentro CLS)  
 Nd = 163.75  
 Mdz= 108.5707  
 Mdy= -.0057

UNITA' DI MISURA:  
 kN; cm; kNm; N/mm2.  
 Ø in mm; deformazioni\*1000.  
 SIMBOLI:  
 S=sigma (tensioni sui materiali);  
 D=deformazioni (epsilon):



TENSIONI NEL CLS:					
ver	Z	Y	Dcls	Scls	Ve
1	-12.5	70.	-.34251	-4.42	si
2	12.5	70.	-.34211	-4.41	si
3	12.5	0.	1.65654	0.	si
4	-12.5	0.	1.65614	0.	si

TENSIONI NEI FERRI:							
fer	Z	Y	Ø (mm)	Af (cm2)	D ferri	S ferri	Ve
1	9.5	67.	16	2.01	-.2565	-91.3	si
2	-9.5	67.	16	2.01	-.2568	-91.36	si
3	9.5	3.	16	2.01	1.57084	314.17	si
4	3.2	3.	16	2.01	1.57073	314.15	si
5	-3.2	3.	16	2.01	1.57063	314.13	si
6	-9.5	3.	16	2.01	1.57053	314.11	si

% ARMAT.: tesa= .46; comp.= .23; tot.= .69

Ripristino Ponte-Tubo  
 VERIFICA SELLA III INTERMEDIA

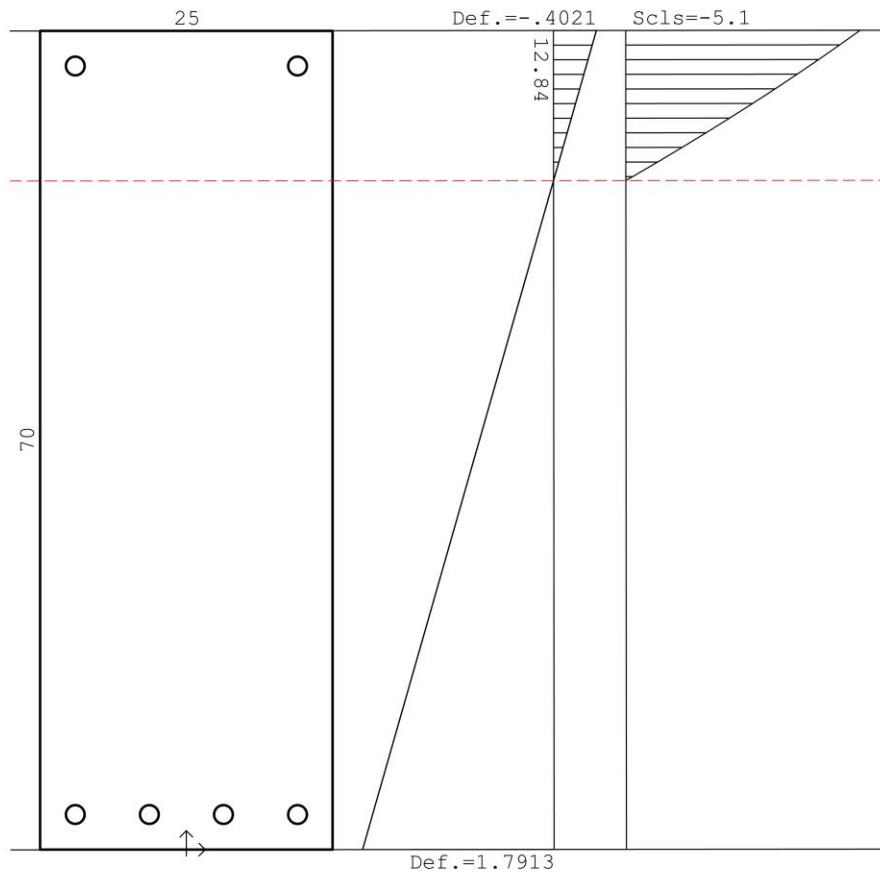
DESCRIZIONI  
 Tipo sezione : RETTANGOLARE  
 Tipo verifica: stato limite ultimo

CARATTERISTICHE DEI MATERIALI  
 Calcestruzzo: Rck= 30.  
 fck= 24.9  
 fcd= 14.11 (.35%)  
 Acciaio lento: Tipo= B450C  
 fyk= 450.  
 ftk= 540.  
 ftd= 469.57 (6.75%)

PIANO DI EQUILIBRIO:  
 eps= muz \* y +muy \* z + lam  
 muz=-3.13238177280708E-05  
 muy= 2.65607318672841E-08  
 lam= 1.79092655421453E-03

SOLLECITAZIONI AGENTI:  
 Nd in z= 0. ; y= 35. (baricentro CLS)  
 Nd = 163.41  
 Mdz= 121.2534  
 Mdy= -.0097

UNITA' DI MISURA:  
 kN; cm; kNm; N/mm2.  
 Ø in mm; deformazioni\*1000.  
 SIMBOLI:  
 S=sigma (tensioni sui materiali);  
 D=deformazioni (epsilon):



TENSIONI NEL CLS:

ver	Z	Y	Dcls	Scls	Ve
1	-12.5	70.	-.40207	-5.1	si
2	12.5	70.	-.40141	-5.1	si
3	12.5	0.	1.79126	0.	si
4	-12.5	0.	1.79059	0.	si

TENSIONI NEI FERRI:

fer	Z	Y	Ø (mm)	Af (cm2)	D ferri	S ferri	Ve
1	9.5	67.	16	2.01	-.30752	-61.5	si
2	-9.5	67.	16	2.01	-.30802	-61.6	si
3	9.5	3.	16	2.01	1.69721	339.44	si
4	-9.5	3.	16	2.01	1.69704	339.41	si
5	-3.2	3.	16	2.01	1.69687	339.37	si
6	9.5	3.	16	2.01	1.6967	339.34	si

% ARMAT.: tesa= .46; comp.= .23; tot.= .69



# Ripristino Ponte-Tubo

## VERIFICA TRAVE NORD

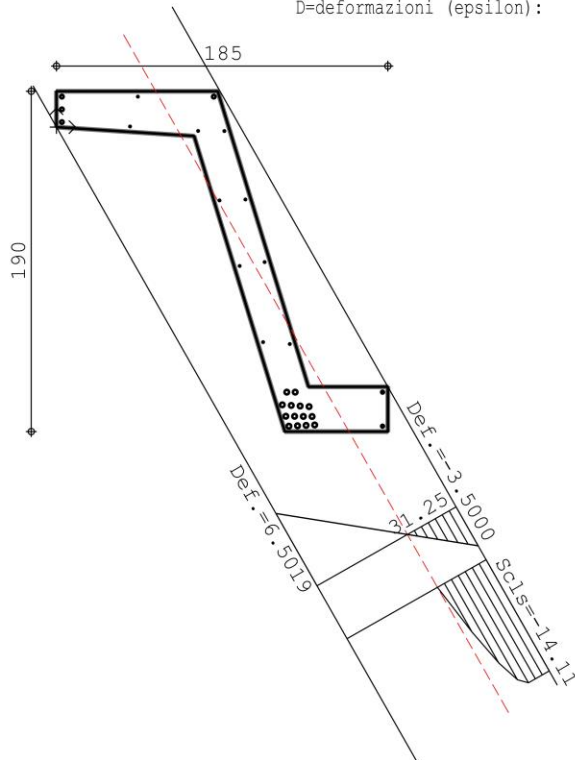
DESCRIZIONI  
 Tipo sezione : a Z sinistra  
 Tipo verifica: stato limite ultimo

CARATTERISTICHE DEI MATERIALI  
 Calcestruzzo: Rck= 30.  
 fck= 24.9  
 fcd= 14.11 (.35%)  
 Acciaio lento: Tipo= B450C  
 fyk= 450.  
 ftk= 540.  
 ftd= 469.57 (6.75%)  
 Fibre : FRMC in PBO  
 Samm= 1036.3  
 n = 12

PIANO DI EQUILIBRIO:  
 eps= muz \* y + muy \* z + lam  
 muz=-5.53239221641092E-05  
 muy=-9.74345843165769E-05  
 lam= 6.50194724484283E-03

SOLLECITAZIONI AGENTI:  
 Nd in z= 99.8; y= -66.9 (baricentro CLS)  
 Nd = -53.3  
 Mdz= 2585.6437  
 Mdy= -58.7463  
 Mzult/Mdz=2585.6437 / 3171.0279 =.8154

UNITA' DI MISURA:  
 kN; cm; kNm; N/mm2.  
 Ø in mm; deformazioni\*1000.  
 SIMBOLI:  
 S=sigma (tensioni sui materiali);  
 D=deformazioni (epsilon):



TENSIONI NEL CLS:						
ver	Z	Y	Dcls	Scls	Ve	
1	0.	0.	6.50195	0.	si	
2	0.	20.	5.39547	0.	si	
3	90.	20.	-3.37364	-14.11	si	
4	140.7	-145.	.81487	0.	si	
5	185.	-145.	-3.50000	-14.11	si	
6	185.	-170.	-2.11838	-14.11	si	
7	127.5	-170.	3.4841	0.	si	
8	76.8	-5.	-7.0441	-8.19	si	

TENSIONI NEI FERRI:										
Fer	Z	Y	Ø (mm)	Af (cm2)	D ferri	S ferri	Ve			
1	3.	17.	20	3.14	5.26914	395.26	si			
2	3.	9.9	20	3.14	5.6616	395.73	si			
3	3.	2.8	20	3.14	6.05413	396.2	si			
4	45.4	17.	8	.5	1.13869	227.74	si			
5	41.	.3	8	.5	2.48567	391.94	si			
6	93.7	-2.2	8	.5	-2.50365	-391.96	si			
7	79.1	-2.1	8	.5	-1.08278	-216.56	si			
8	87.8	17.	20	3.14	-2.99166	-392.54	si			
9	105.4	-40.4	8	.5	-1.53378	-306.76	si			
10	91.	-40.9	8	.5	-.09775	-19.55	si			
11	116.2	-75.4	8	.5	-.64725	-129.45	si			
12	102.2	-77.5	8	.5	-.83121	166.24	si			
13	130.2	-121.1	8	.5	-.51451	102.9	si			
14	115.3	-120.1	8	.5	1.91134	382.27	si			
15	182.	-148.	16	2.01	-3.04321	-392.6	si			
16	182.	-167.	16	2.01	-1.99205	-391.35	si			
17	144.1	-166.3	26	5.31	1.86201	332.4	si			
18	139.3	-166.6	26	5.31	2.14205	391.53	si			
19	134.5	-166.8	26	5.31	2.62208	392.1	si			
20	129.7	-167.	26	5.31	3.10212	392.67	si			
21	142.7	-161.6	26	5.31	1.54237	308.47	si			
22	137.8	-161.6	26	5.31	2.01532	391.37	si			
23	132.9	-161.5	26	5.31	2.48812	391.94	si			
24	128.	-161.4	26	5.31	2.96098	392.5	si			
25	141.	-156.1	26	5.31	1.40235	280.47	si			
26	136.	-155.8	26	5.31	1.86796	373.59	si			
27	131.	-155.4	26	5.31	2.33357	391.75	si			
28	126.	-155.1	26	5.31	2.79918	392.31	si			
29	133.2	-148.	26	5.31	1.70926	341.85	si			
30	128.5	-148.	26	5.31	2.16584	391.55	si			

FASI DI APPLICAZIONE FIBRE:  
 | fase |  
 | 1 | dopo carichi permanenti |

TENSIONI MASSIME FIBRE (fase 0 -> applic. a sez. indeformata):											
fib	fas	Z1	Y1	Z2	Y2	s (mm)	A (cm2)	D cls	D fibra	S fibra	Ve
1	1	0.	0.	0.	20.	10.	20.	.2617	.2617	102.44	si
2	1	0.	20.	90.	20.	10.	90.	.1768	.1768	69.22	si
3	1	90.	20.	140.7	-145.	10.	172.61	-.1157	-.1157	45.32	si
4	1	140.7	-145.	185.	-145.	10.	44.3	-.1157	-.1157	45.32	si
5	1	185.	-145.	185.	-170.	10.	25.	-.0178	-.0178	-6.98	si
6	1	185.	-170.	127.5	-170.	10.	57.5	-.2932	-.2932	114.8	si
7	1	127.5	-170.	76.8	-5.	10.	172.61	-.2932	-.2932	114.8	si
8	1	76.8	-5.	0.	0.	10.	76.96	.2617	.2617	102.44	si

% ARMAT.: tesa= 1.3; comp.= .15; tot.= 1.45

# Ripristino Ponte-Tubo

## VERIFICA TRAVE SUD

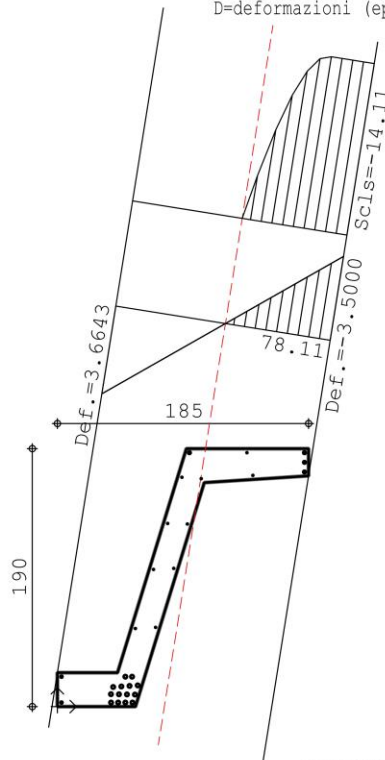
DESCRIZIONI  
 Tipo sezione : a Z destra  
 Tipo verifica: stato limite ultimo

CARATTERISTICHE DEI MATERIALI  
 Calcestruzzo: Rck= 30.  
 fck= 24.9  
 fcd= 14.11 (.35%)  
 Acciaio lento: Tipo= B450C  
 fyk= 450.  
 ftk= 540.  
 ftd= 469.57 (6.75%)  
 Fibre : FRMC in PBO  
 Samm= 1036.3  
 n = 12

PIANO DI EQUILIBRIO:  
 eps= muz \* y + muy \* z + lam  
 muz= 7.05526472495483E-06  
 muy=-4.42607301227047E-05  
 lam= 3.48794960344304E-03

SOLLECITAZIONI AGENTI:  
 Nd in z= 85.2; y= 103.1 (baricentro CLS)  
 Nd = -55.14  
 Mdz= 3819.0214  
 Mdy= 2578.3703  
 Myult/Mdy=2578.3703 / 101.6268 = 25.3709683529586

UNITA' DI MISURA:  
 kN; cm; kNm; N/mm2.  
 Ø in mm; deformazioni\*1000.  
 SIMBOLI:  
 S=sigma (tensioni sui materiali);  
 D=deformazioni (epsilon):



ver	Z	Y	Dcls	Scls	Ve
1	0.	0.	3.48795	0.	si
2	0.	25.	3.66433	0.	si
3	44.3	25.	1.70358	0.	si
4	95.	190.	.62368	0.	si
5	185.	190.	-3.35979	-14.11	si
6	185.	170.	-3.50000	-14.11	si
7	108.2	165.	-1.13694	-1.87	si
8	57.5	0.	.94296	0.	si

fer	Z	Y	Ø (mm)	Af (cm2)	D ferri	S ferri	Ve
1	182.	187.	20	3.14	-3.24817	-392.85	si
2	182.	179.9	20	3.14	-3.29822	-392.91	si
3	182.	172.8	20	3.14	-3.34828	-392.97	si
4	139.6	187.	8	.5	-1.37187	-274.37	si
5	144.	170.3	8	.5	-1.68251	-336.5	si
6	91.7	168.9	8	.5	.62266	124.53	si
7	105.9	167.9	8	.5	-.01674	-3.35	si
8	95.7	134.5	8	.5	-.20153	40.31	si
9	81.2	134.9	8	.5	.84544	169.09	si
10	85.6	101.8	8	.5	.41555	83.11	si
11	70.8	101.	8	.5	1.06712	213.42	si
12	72.3	58.3	8	.5	.70007	140.01	si
13	57.4	57.5	8	.5	1.35188	270.38	si
14	97.2	187.	20	3.14	.50439	100.88	si
15	3.	22.	16	2.01	3.51038	393.16	si
16	3.	3.	16	2.01	3.37633	393.	si
17	39.5	3.	26	5.31	1.76095	352.19	si
18	44.8	3.	26	5.31	1.52805	305.61	si
19	50.	3.	26	5.31	1.29515	259.03	si
20	55.3	3.	26	5.31	1.06225	212.45	si
21	39.4	9.1	26	5.31	1.80738	361.48	si
22	45.3	9.	26	5.31	1.54621	309.24	si
23	51.2	9.	26	5.31	1.28504	257.01	si
24	57.1	8.9	26	5.31	1.02386	204.77	si
25	41.1	14.4	26	5.31	1.77276	354.55	si
26	47.1	14.8	26	5.31	1.50844	301.69	si
27	53.1	15.2	26	5.31	1.24412	248.82	si
28	59.2	15.6	26	5.31	.97976	195.95	si
29	50.	22.	26	5.31	1.42827	285.65	si
30	55.	22.	26	5.31	1.21051	242.1	si

FASI DI APPLICAZIONE FIBRE:  
 fase |  
 1 | dopo carichi permanenti |

TENSIONI MASSIME FIBRE (fase 0 -> applic. a sez. indeformata):											
fib	fas	Z1	Y1	Z2	Y2	s (mm)	A (cm2)	D cls	D fibra	S fibra	Ve
1	1	0.	0.	0.	25.	10.	25.	-.0569	-.0569	-22.28	si
2	1	0.	25.	44.3	25.	10.	44.3	-.1859	-.1859	72.79	si
3	1	44.3	25.	95.	190.	10.	172.61	-.1859	-.1859	72.79	si
4	1	95.	190.	185.	190.	10.	90.	.3403	.3403	133.22	si
5	1	185.	190.	185.	170.	10.	20.	.4844	.4844	189.65	si
6	1	185.	170.	108.2	165.	10.	76.96	.4844	.4844	189.65	si
7	1	108.2	165.	57.5	0.	10.	172.61	.4921	.4921	192.68	si
8	1	57.5	0.	0.	0.	10.	57.5	.4921	.4921	192.68	si

% ARMAT.: tesa= 1.29; comp.= .17; tot.= 1.45